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# FOUNDATION REPORT

Supplement Number 6

Local Flood Protection Project

Southwestern Jefferson County, Kentucky

SECURITY CLASSIFICATION OF THIS PAGE (Then Date Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
T. REPORT NUMBER	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
ORLCD-1-84	1215-12140 388	
4. TITLE (and Subtitle)		5. TYPE OF REPORT & PERIOD COVERED
Foundation Report, Supplement No. 6, Flood Protection, Section 4, Ohio River, Southwest Jefferson County, Kentucky  USE TITIE ON 1473		Final
		6. PERFORMING ORG. REPORT NUMBER
7. AUTHOR(a)		S. CONTRACT OR GRANT NUMBER(*)
Resident Engineer	ı	
Gary Fitzgerald		DACW27-83-C-0003
5. Performing organization name and address		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
U.S. Army Corps of Engine	ers	
Louisville Resident Offic		
Louisville, Kentucky 402		
11. CONTROLLING OFFICE NAME AND ADDRESS  As Above		12. REPORT DATE
		April 1984
		13. NUMBER OF PAGES
14. MONITORING AGENCY NAME & ADDRES	S/II different from Controlline Office)	98 15. SECURITY CLASS. (of this report)
U.S. Army Corps of Engineers		Unclassified
Louisville District Office		ouctassitied
Construction Division, P.O. Box 59		15e, DECLASSIFICATION/DOWNGRADING
Louisville, Kentucky 40201		SCHEDULE
16. DISTRIBUTION STATEMENT (of this Repo	ort)	

17. DISTRIBUTION STATEMENT (of the obstract entered in Block 20, If different from Report)

Approved for public release; distribution unlimited.

IS. SUPPLEMENTARY NOTES

SELECTE APR 2 3 1984

19. KEY WORDS (Continue on reverse side if necessary and identity by block number)
Ohio River
Jefferson County, Kentucky
Flood Protection

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26. ABSTRACT (Castless on reverse olds If necessary and Identify by block number)

This report is a continuation of a series of reports covering the foundation work for levee construction around the southwest portion of Jefferson County, Kentucky. The report contains a narrative, drawings and photographs of the conditions encountered and the methods employed to perform the contract requirements.

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Levee

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Unclassified

FOUNDATION REPORT

SUPPLEMENT NO. 6

SOUTHWESTERN JEFFERSON COUNTY, KENTUCKY

LOCAL FLOOD PROTECTION PROJECT

CONTRACT NO. DACW27-83-C-0003

CONSTRUCTION SECTION 4 LEVEE AND WALL
STATION 735+00 to STATION 868+00

**APRIL 1984** 

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Volume I - Main Line Station  $775+50^{\frac{1}{2}}$  to Station 867+00

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#### INTRODUCTION

- 1-01 Location of Section 4. Section 4, the fifth reach to be constructed, extends from Section 2 at Station 735+00 southward to a point of tie-in with the embankment of Dixie Highway (Station 868+00) just north of the intersection of Dixie Highway and Kentucky State Highway 44. Actual construction on the north end of this reach began at Station 774+65 at a point of tie-in with the railroad embankment on Louisville Gas and Electric Company Mill Creek Generating Station - property. Between Stations 735+00 and 774+65, the Louisville Gas and Electric Company Mill Creek Generating Station and the railroad embankment provide the required protection. Plate No. 1 shows this section in relation to other construction sections and its position in the total project. Plate Numbers 2, 3, 4 and 5 show the location of Section 4 in detail. Section 4 includes approximately 8,800 feet of earth levee, 1,550 feet of concrete wall, two diversion sewers, two gatewells, four closure structures (Dixie Highway, Houk Lane, Flintkote Access Road, Illinois Central Gulf Railroad).
  - 1-02 <u>Contractors</u>. The prime contractor for construction of Section 4 was Renshaw Construction Company, Inc. of Madisonville, Kentucky. Mr. Elvis H. Butler, Jr., Vice President was the home office principal directly responsible for the project. Mr. Kenny Wigginton was Project Superintendent responsible for onsite operations through August 1983; Mr. Mike Kolstad was Project Superintendent for onsite operations from September 1983 through the time that this supplement was written.

Renshaw Construction Company, Inc. constructed all earth levee and earth embankments for access roads on Section 4.

Firms that subcontracted work on this section of the project and the types of work subcontracted are as follows:

- Concrete Wall, Closure Structures, Gatewells and Storage Vaults T. H. Ballard Construction, Inc. 1474 South Floyd Street Louisville, Kentucky 40208
- b. Toedrain, Riprap, Culvert Pipes, Diversion Sewers and Paved Ditch - T and C Contracting Co. 14201 Bohannon Lane Louisville, Kentucky 40272
- c. Bored Encasement Pipe and Jacked Pipe - Van Meter Construction Co. 790 Westland Drive Lexington, Kentucky 40504

- d. Reinforcing Steel Centen Steel Erectors, Inc.
  Route 2, Box 230B
  Manchester, Tennessee 37355
- e. Steel Sheet Piling RAM Engineering & Construction, Inc.
  P.O. Box 35160
  Louisville, Kentucky 40232
- f. Chain Link Fence Cardinal Fence Company
  4615 Illinois Avenue
  Louisville, Kentucky 40213
- g. Roof Deck and Insulation on Storage Vaults - Triangle Industries 4626 Illinois Avenue Louisville, Kentucky 40213
- h. Masonry on Storage Vaults Cardinal Construction Company
  2409 West Market Street
  Louisville, Kentucky
- Service Doors on Storage
   Vaults Overhead Door Company of Louisville
   3909 Oaklawn
   Louisville, Kentucky 40219
- j. Seeding Southern Contractors, Inc.
   208 Dishman Lane
   Bowling Green, Kentucky 42101
- k. Stone and Asphalt Murray Company, Inc.
  P.O. Box 23410
  Anchorage, Kentucky 40223
- m. Excavation and Backfill for Concrete Wall and Structures Breslin Company, Inc.
  P.O. Box 35582
  Louisville, Kentucky

WM. G. Scott Excavating 4000 Camp Ground Road Louisville, Kentucky

V. J. Dermody 4613 Bittersweet Road Louisville, Kentucky 40218 1-03 <u>Contract Supervision</u>. Government personnel responsible for onsite administration of the Section 4 work was:

Mr. Gary V. Fitzgerald - Resident Engineer

## FOUNDATION EXPLORATIONS

- 2-01 Subsurface Investigations Prior to Construction. Investigations were made for the levee and floodwall using drive sampling, Denison, NX core, power auger and hand auger methods. The borrow areas were investigated mainly by the use of hand augers. Boring locations are presented in Design Memorandum No. 1 on Plates 1B-29, 33-35, 37-42 and 54-61. Graphic logs are presented on Plates 63 through 78. Initial drilling to determine the scope of work was begun in 1965 and was accomplished by contract drilling. This was supplemented by District Drilling in 1966, 1970 and 1971. A portion of these logs are presented on Plate Numbers 6, 7 and 8 of this supplement. Locations of the borings are shown on Plates 2, 3, 4 and 5 of this supplement.
  - 2-02 Foundation Investigation During Construction.

    Investigations during construction consisted of visual inspection of the foundation and inspection trench prior to embankment placement. A required inspection trench was excavated between Station 774+65 and Station 825+00, Station 845+66 and Station 856+50, Station 860+40 and Station 867+75. Foundation conditions were also visually inspected after excavation and prior to construction of the concrete T-Wall and closure structures.

**GEOLOGY** 

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3-01 Engineering Characteristics of Overburden Materials.
No specific foundation soil engineering studies were conducted within this reach of levee. The borings taken prior to construction show the soil classifications encountered. The silts, clays and sands were similar to those from the adjacent levee Sections 2 and 5 where extensive studies were carried out. (See Foundation Report Supplement Numbers 2 and 4 or Supplement No. 2 to Design Memorandum No. 1).

A representative number of borings taken in Section 4 are shown on Plates 6 through 8 of this report.

## 3-02 T-Wall.

- a. General. The T-wall stability analysis and test data are presented in Design Memorandum No. 1 and summarized in Supplement No. 2 to Design Memorandum No. 1. Strength values obtained from tests on soil from boring U-501A and U-502 were used in analyzing the wall.

  Both structural and sliding stability analyses were done using a GE 225 computer. Program number 41-G1-H201 was used.
- b. Structural Stability Analysis. Plate 79 of Design Memorandum No. 1 shows the dimensions and forces used in the manual check of the computer. The calculations are shown on Plates 80 through 83 of Design Memorandum No. 1. The resultant from loading number 1 is 0.033 foot in from the quarter point and the resultant from loading number 2 is 0.675 foot in from the one-third point, thus satisfying the conditions for the point of action of resultants set forth in EM 1110-2-2501.

The toe pressure, creep ratio and estimated horizontal movement were also calculated and the calculations are shown on Plates 84 and 85 of Design Memorandum No. 1. All of the above items were within the allowable limits established in EM 1110-2-2501.

c. <u>Sliding Stability Analysis</u>. For simplicity of design, a flat failure plane was assumed. The method of analysis is shown in Figure 5-10 of EM 1110-2-2501. In computing the uplift along the failure plane, a straight line assumption was used in lieu of a flow net with full flood head assumed acting at the bottom of the key with the intersection at the failure plane and ground surface being the point of zero potential. This assumption has been proven to be on the conservative side.

The manual calculations made to check the computer results for sliding stability are shown on Plates 86 and 87 of Design Memorandum No. 1. The lowest factor of safety obtained for the "Q" case was 5.17 and the lowest factor of safety for the "R" case was 3.01.

Plates 88 and 89 of Design Memorandum No. 1 show the required shearing strength curves obtained by using a factor of safety of 1.0 and 1.5+2c. These curves were based on the test values obtained from holes U-501A and U-502 since no values were available on borrow areas at the time of the analysis. Tests on the borrow areas yielded higher strengths than the values used. Therefore, it was not considered necessary to rerun the analysis since the new values would raise the factor of safety.

4-01 Excavation Grades. The contract plans and specifications call for the levee to be built essentially on existing ground after stripping whatever organic materia? existed except for the special foundation excavation specified between Stations 825+00 and 833+00. Very little additional excavation for unsuitable material was necessary. The areas that were undercut occurred at locations where the drainage in the existing area was poor or where top soil and roots remained in the foundation after required excavation grades were reached. These areas were located under both the earth embankment portion of the levee and the concrete T-wall section.

A typical cross section of the levee is included on Plate Number 4 showing limits of excavation. Typical sections of T-wall are shown on Plate Number 9.

Plate Numbers 10 and 11 show the profile of the wall and closure structures. The numbering of the wall and closure monoliths on these plates will reference locations of foundation photographs included in this supplement and undercuts of unsuitable material discussed in paragraph 4-02.

- 4-02 <u>Method of Excavation</u>. The following paragraphs describe the various mehtods used to excavate different features of this work.
- 4-02.1 Stripping. Stripping excavation involved removal of organic material from beneath the levee embankment limits plus an additional five feet outside the toe. This excavation was done using motorized, rubber tired scrapers. The average depth of removal was six inches except between Stations 813+00 and 821+50 where the depth of top soil and dumped overburden varied from one to four feet deep as was indicated on the boring logs. There was a wet swampy area covered with a growth of trees and cattails (reedy marsh plants) between Stations 775+50 and 778+50. This area contained between three and five feet of very wet soft material that had to be removed by tracked backhoe before a suitable solid foundation material was reached. After the unstable material in this area was removed, the foundation was treated by conventional equipment in the normal manner. Except for T-wall monoliths 49 and 50, the foundations under the concrete wall and closure structures were determined to be suitable after excavating to required grades. At T-wall monoliths 49 and 50 a dark black organic material containing roots was encountered after excavation to required grades; removal of this material and refill with suitable foundation material was required. Excavation of the unsuitable foundation material for the levee and wall between Stations 825+00 and 833+00 was accomplished with suitable foundation material being encountered at the elevations shown in the contract documents.

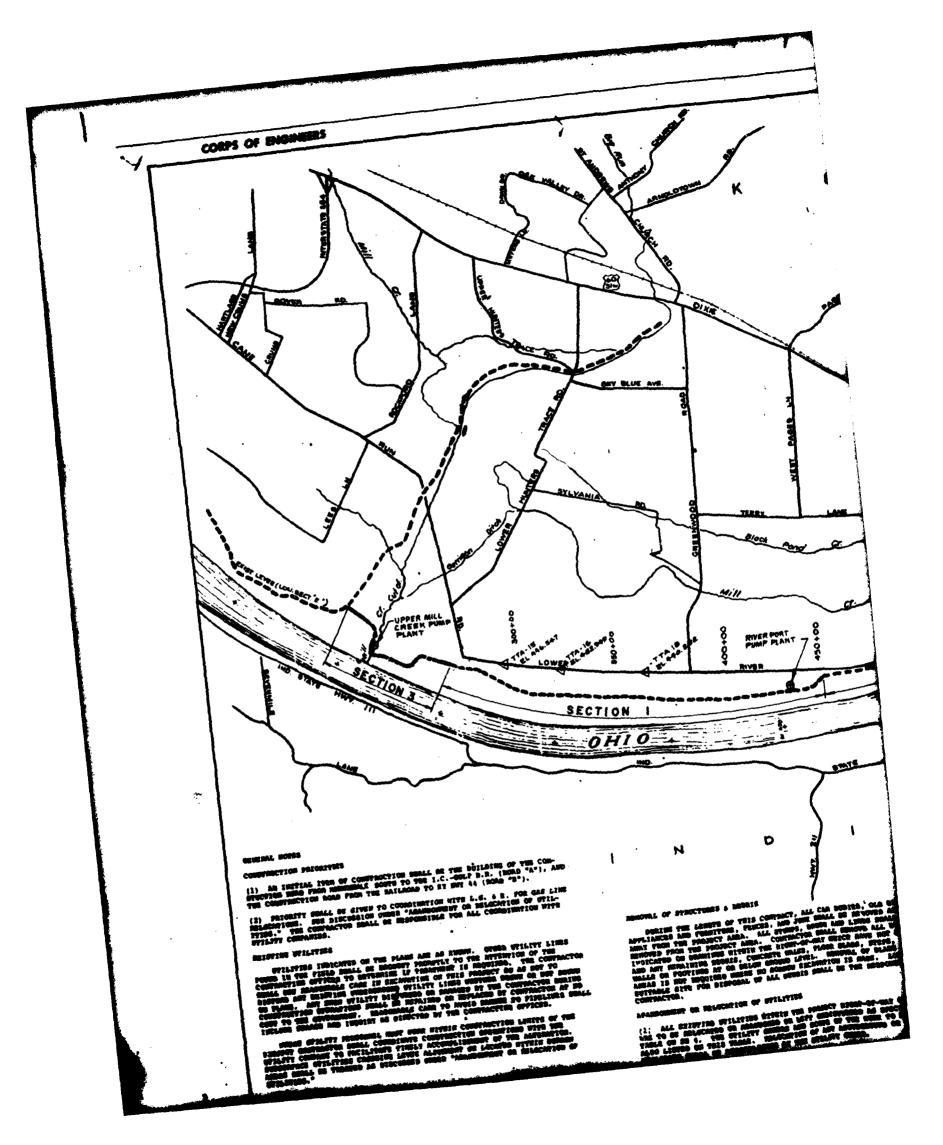
- 4-02.2 <u>Inspection Trench</u>. An inspection trench was excavated in the levee foundation as required by the contract documents. The trench was excavated by rubber tired scrapers assisted by push tractor. The trench was excavated to sufficient width to permit the use of dozers and rollers to recompact the material after the subsurface ground conditions were inspected.
- Foundation Preparation. The earth embankments for this section of the project were founded on essentially the same type of foundation material throughout the entire length. Accordingly the - foundation preparation procedures were basically consistent for all reaches of the earth levee. The preparation consisted of thoroughly breaking the foundation soil to a depth of six inches by using a construction disc, bringing the in situ material to the proper moisture content and recompacting the material with six passes of an approved roller. After this operation was performed to the satisfaction of the Government, embankment placement proceeded in accordance with the contract requirements. This procedure also applied to the in situ material encountered after removal of unsuitable foundation material between Stations 825+00 and 828+75 and after removal of the unstable material between Stations 775+50 and 778+50. In those areas where the concrete wall was constructed, the foundation required no extra or unusual treatment except monoliths 49 and 50 where unsuitable foundation material was removed and replaced with suitable material compacted by hand tampers. The special foundation excavation required between Stations 828+75 and 833+00 was accomplished to the specified elevation where suitable foundation material was encountered. Suitable compacted backfill was then placed to the lines and grades necessary to construct the wall sections. In all other areas the foundation was excavated to the lines and grades specified, the foundation was inspected and placement of concrete proceeded.
  - Station 852+54 outside the limits of the levee but within the limits of Access Road A, a four foot diameter hole 35 feet deep was encountered during construction of Access Road A. The hole was cased with concrete pipe. Holes of this type are common in the area; they are dug to a depth where river gravel is encountered and effluent from septic tanks is piped into the hole instead of using lateral fields. Based upon guidance furnished by personnel from Geotechnical Branch, the hole was filled with sand to within three feet of original ground. The sand was flooded as it was placed in the hole. A three foot thick compacted clay cap was placed on the sand and the road embankment was placed as required by the contract. A photograph of the hole is included in the photographs at the end of this supplement (Station 852+84 L).

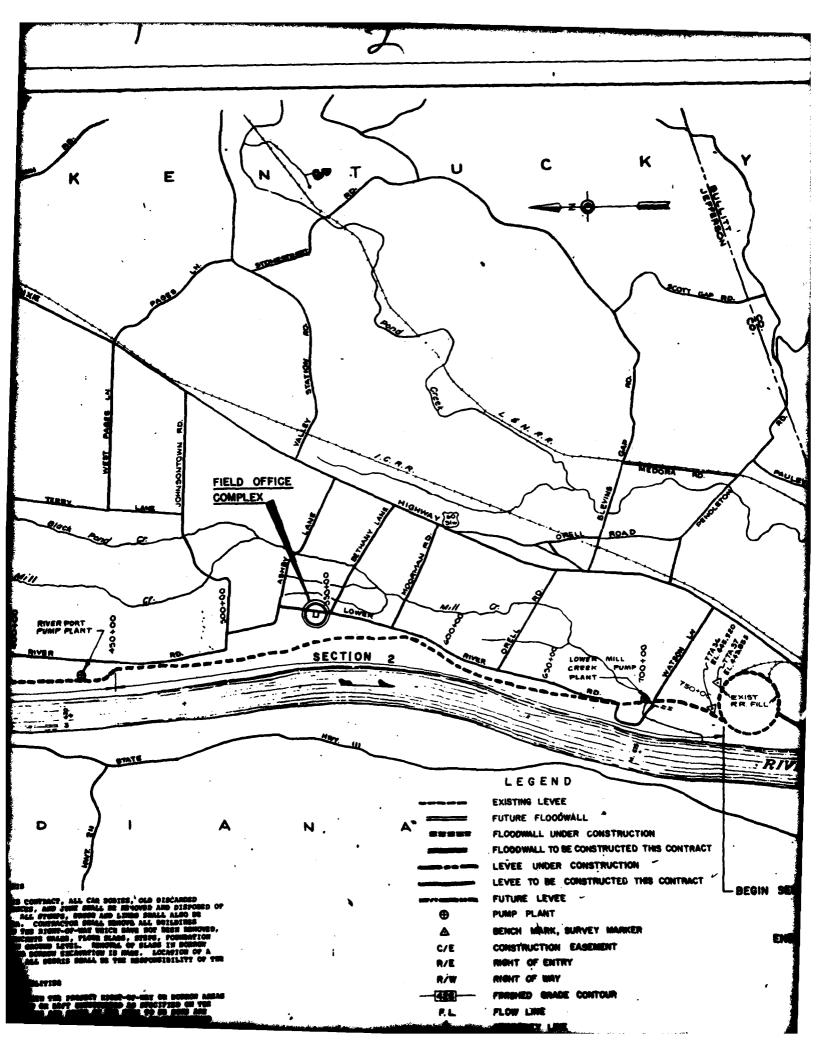
## **GATEWELLS**

- 5-01 Locations of Gatewells. There were two gatewells constructed on this section of levee to provide drainage through the protection during periods when flooding is not occurring. A gatewell was constructed integrally with T-wall monolith 32 at Station 835+32.5 and was connected to an existing 36-inch diameter pipe under Dixie Highway. Location of this gatewell can be identified on Plate No. 10 included with this supplement. The other gatewell constructed on this section of levee was located at Station 767+12<sup>±</sup> on the centerline of survey traverse along the railroad embankment. This gatewell was constructed on an existing 48-inch diameter pipe that provided drainage under the existing railroad embankment which is an integral part of the protection project as discussed in paragraph 1-01. Location of this gatewell in reference to the project can be identified on Plate No. 12 with this supplement.
  - 5-02 <u>Deviations from Planned Conditions</u>. There were no deviations from planned conditions in construction of the gatewells. Excavations were made to the planned lines and grades; suitable foundation material was encountered and construction of the gatewells proceeded.

## POSSIBLE FUTURE PROBLEMS

- 6-01 Conditions That Could Produce Problems. There were no founding conditions encountered that are anticipated to produce future problems. The only conditions that deviated from planned conditions were discussed in paragraphs 4-02.1 and 4-04.
- 6-02 <u>Recommended Observations</u>. Observations should be made immediately after flood situations where water has been against the levee for indications of sliding.





#### GENERAL HOUSE

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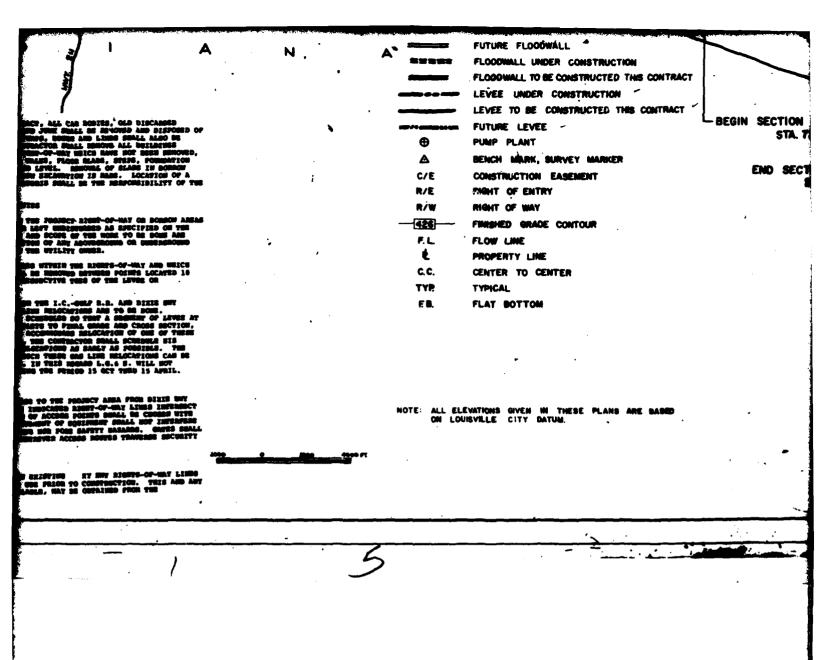
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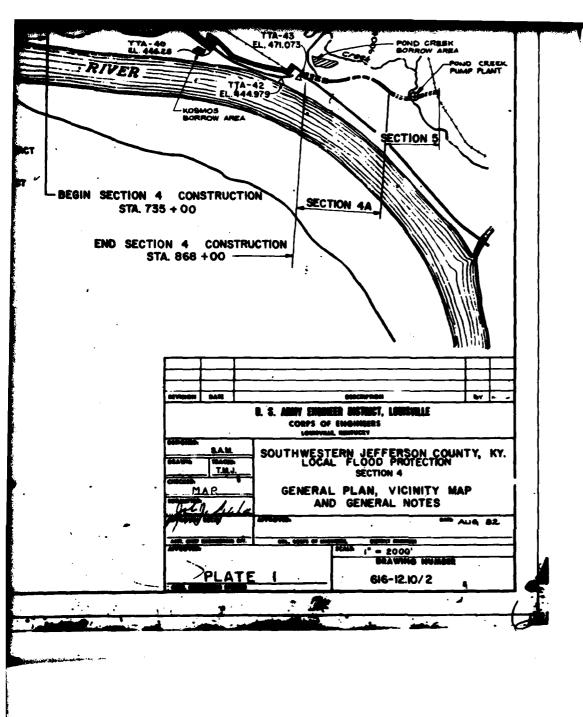
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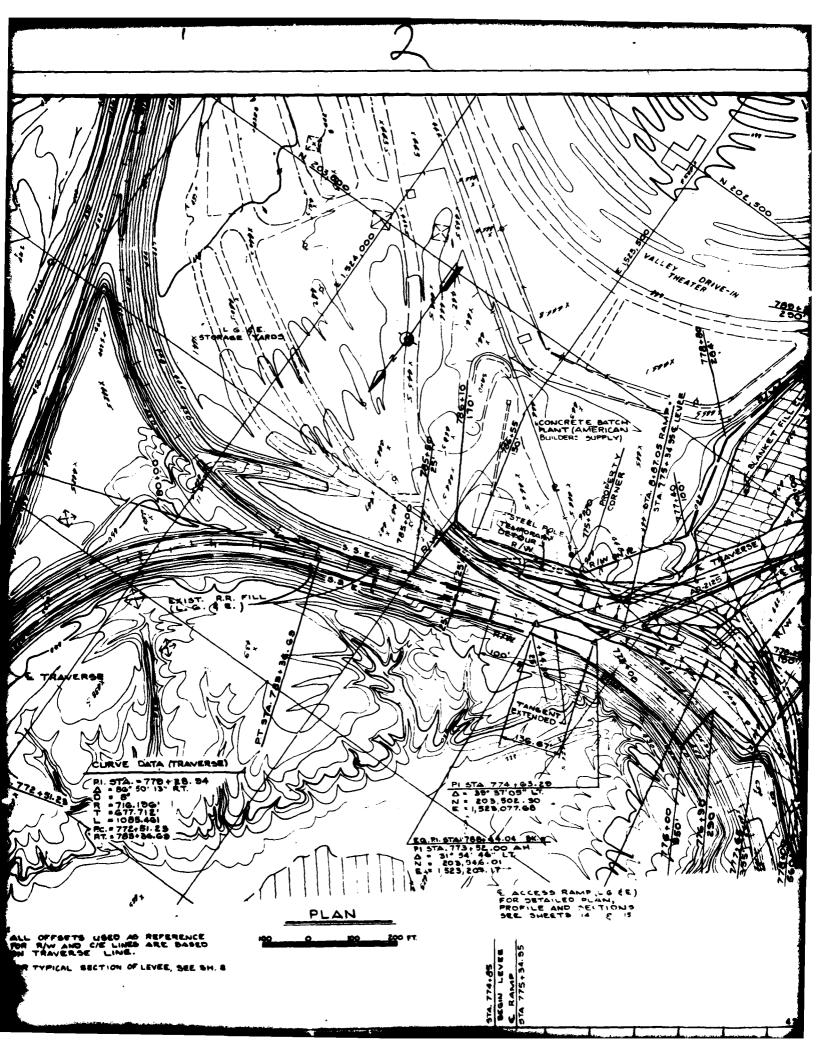
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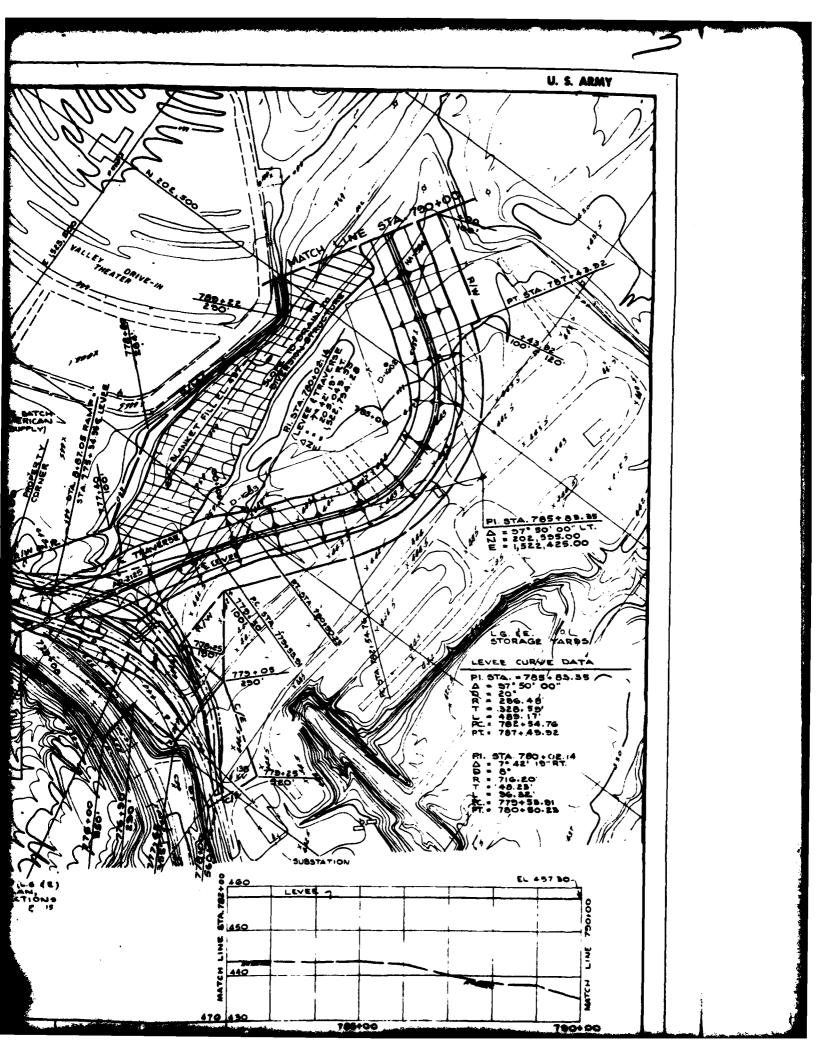
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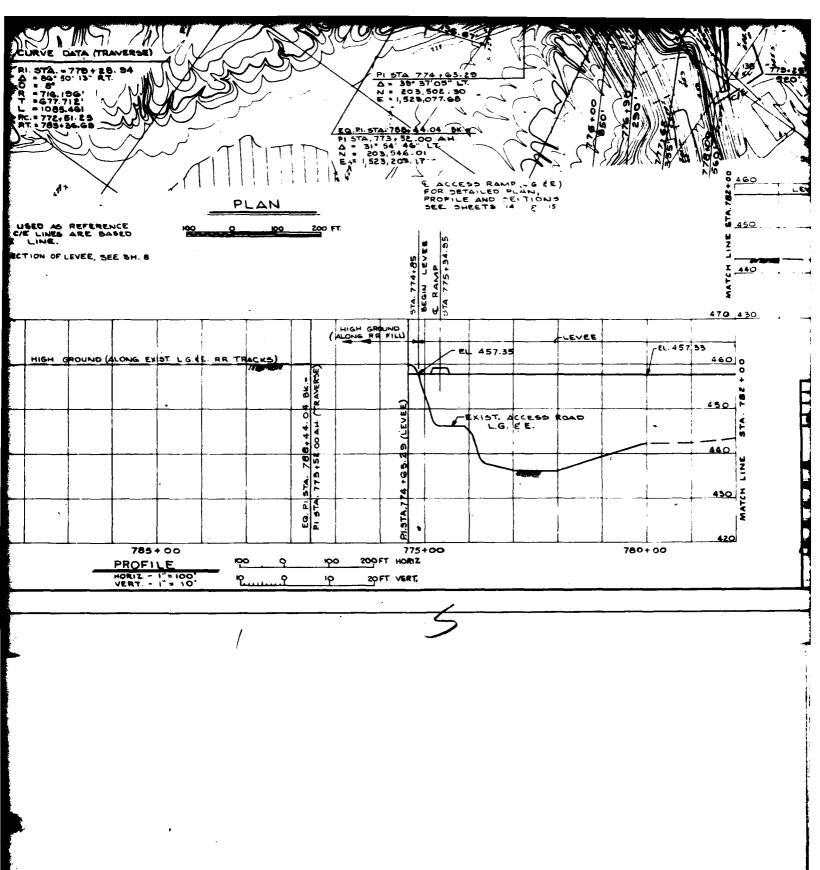




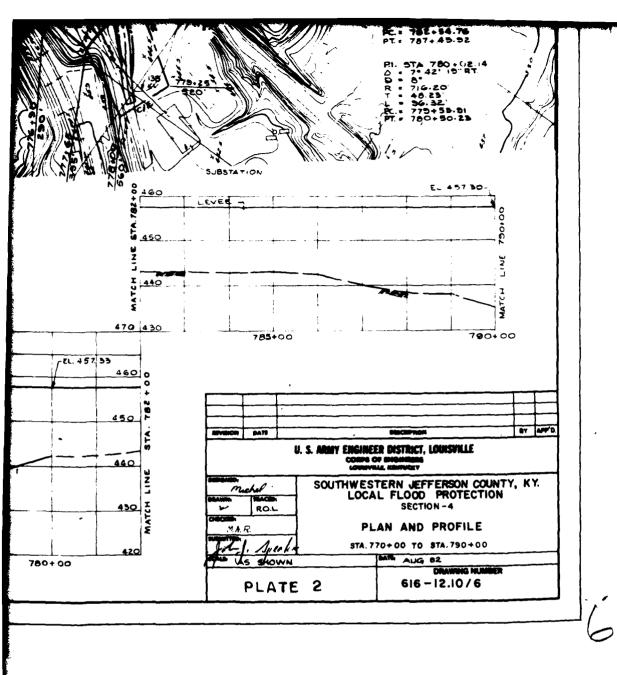


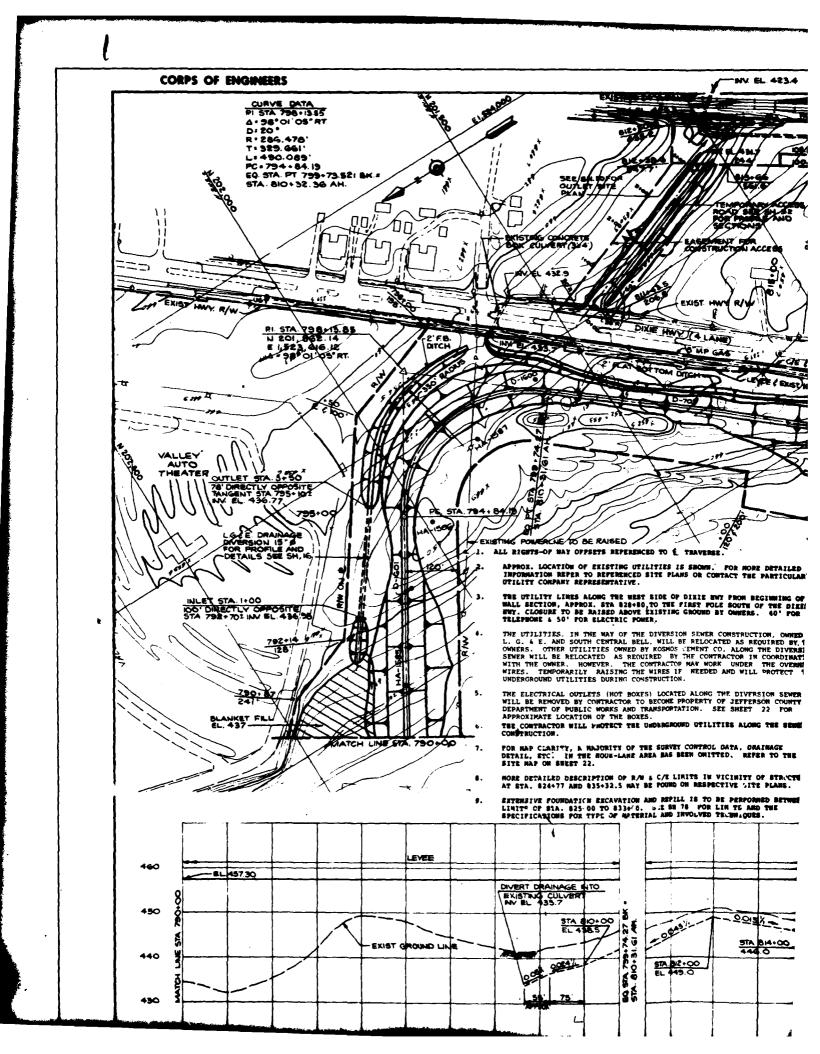
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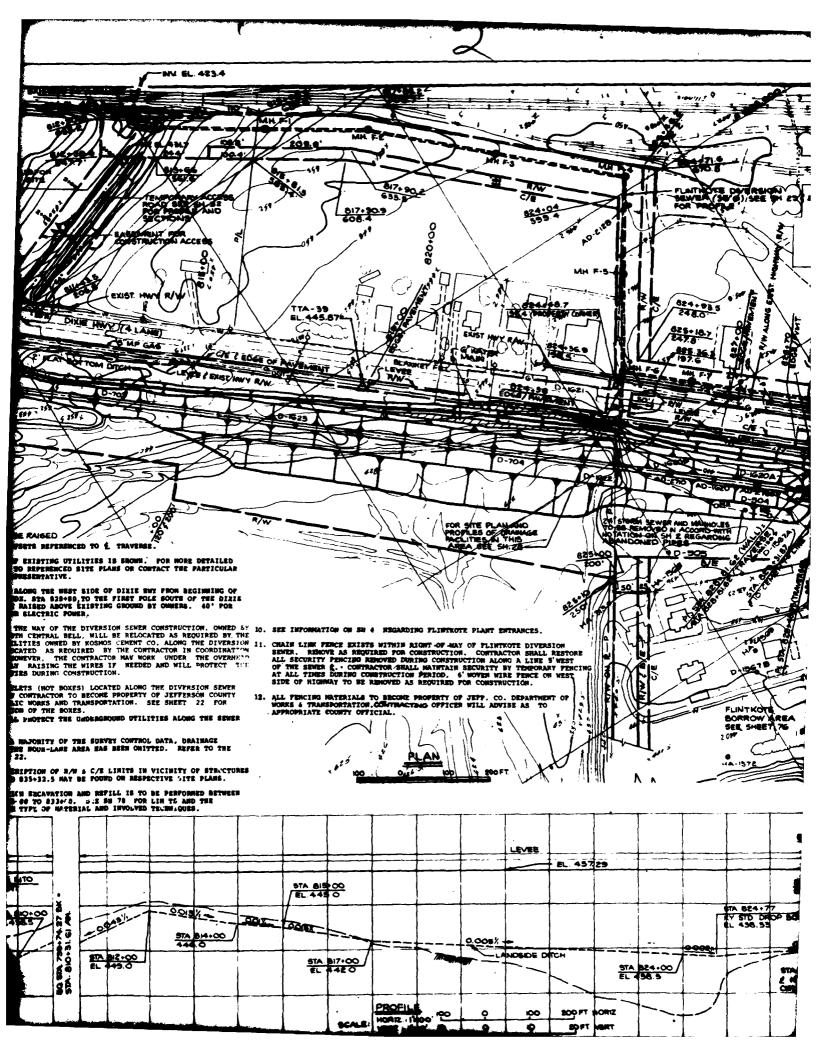
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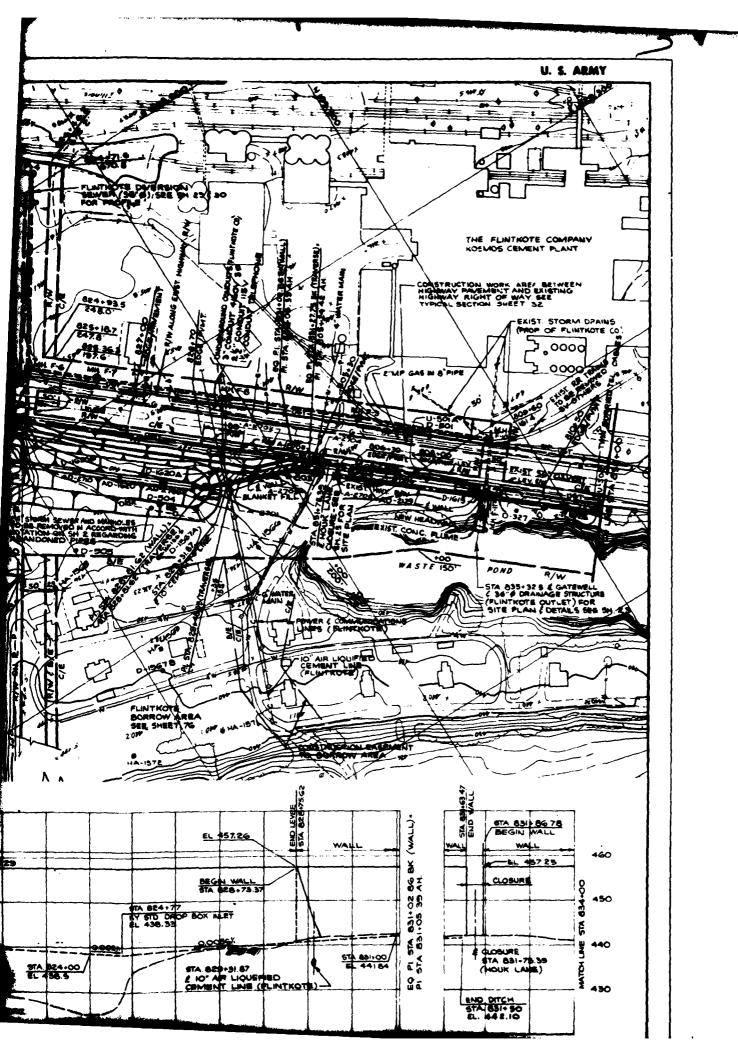


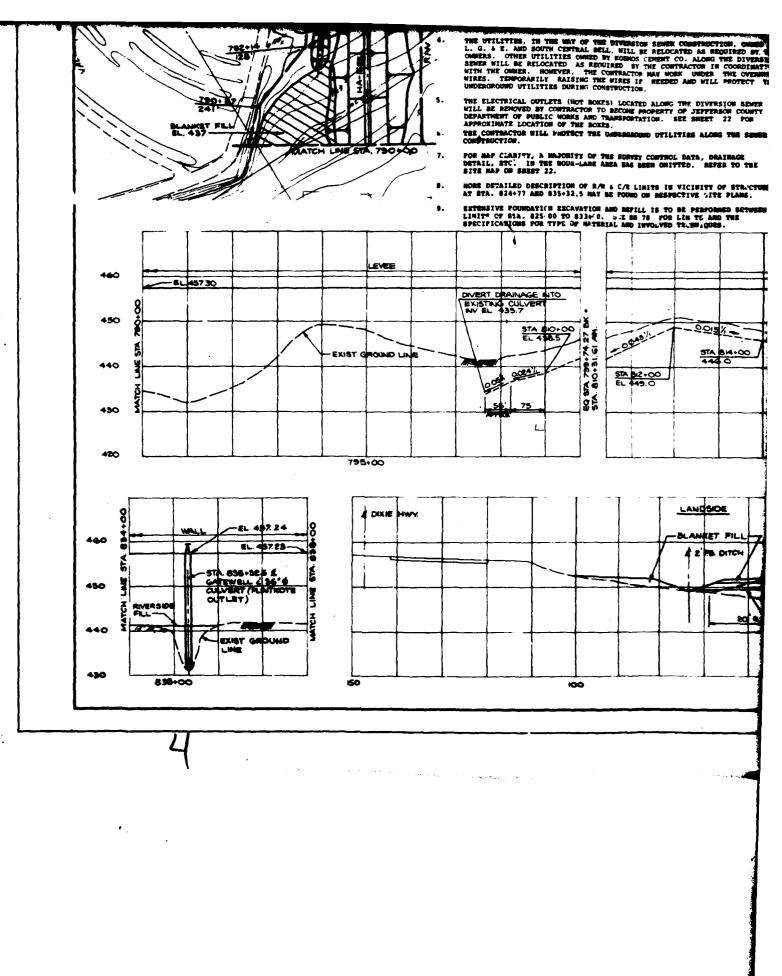
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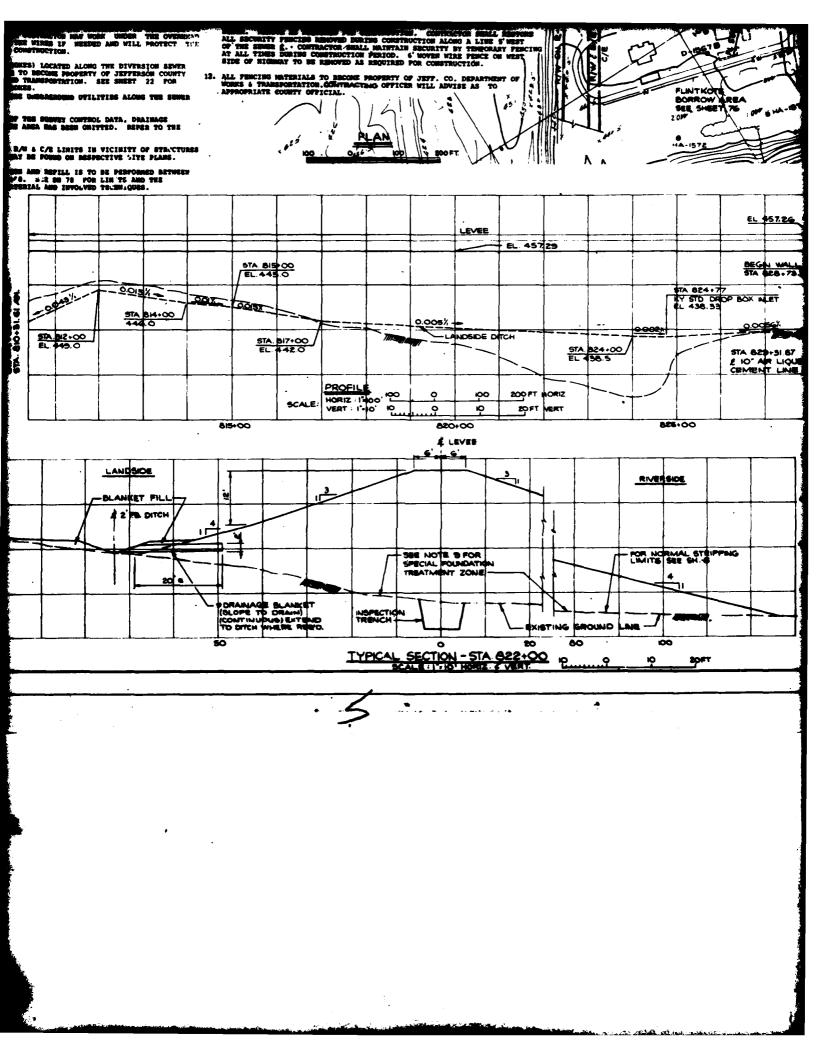


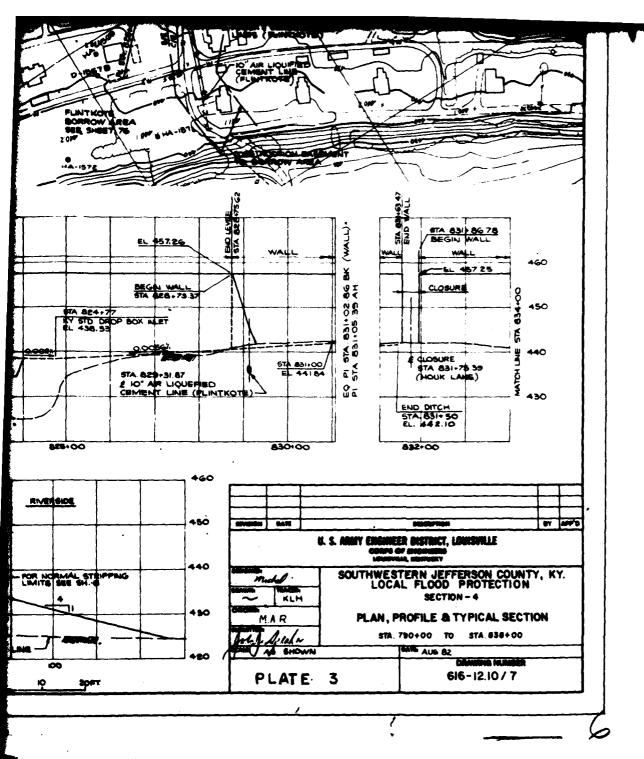


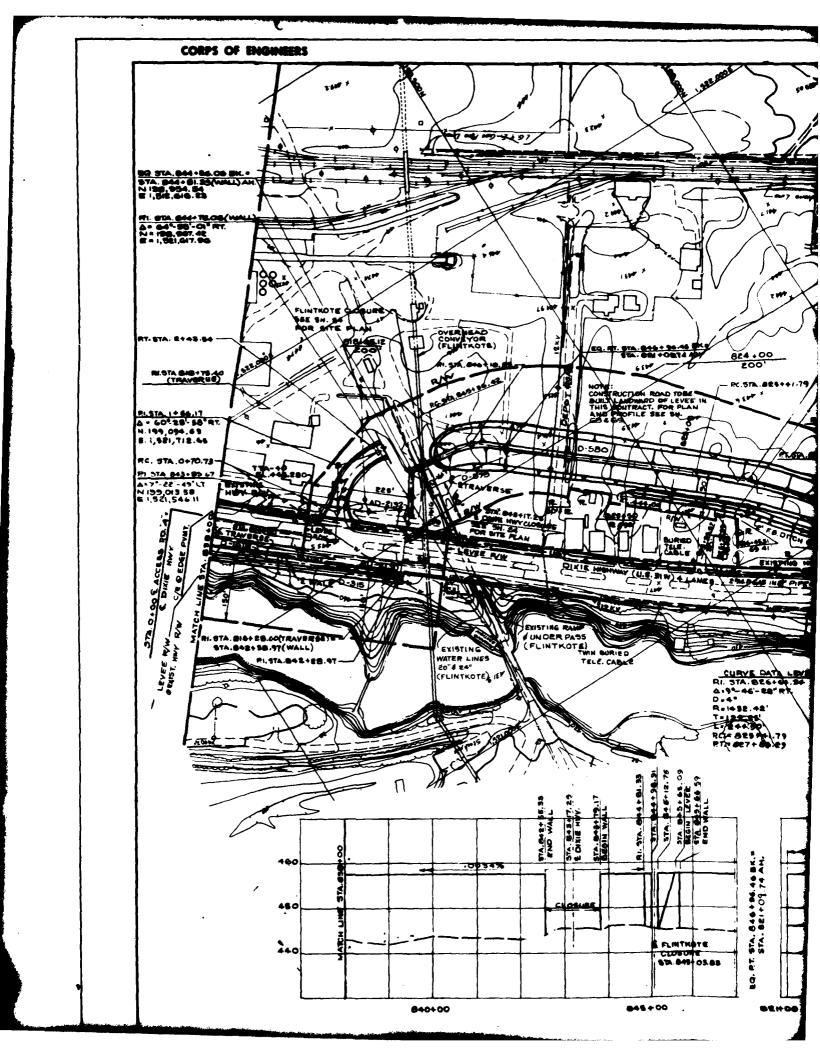


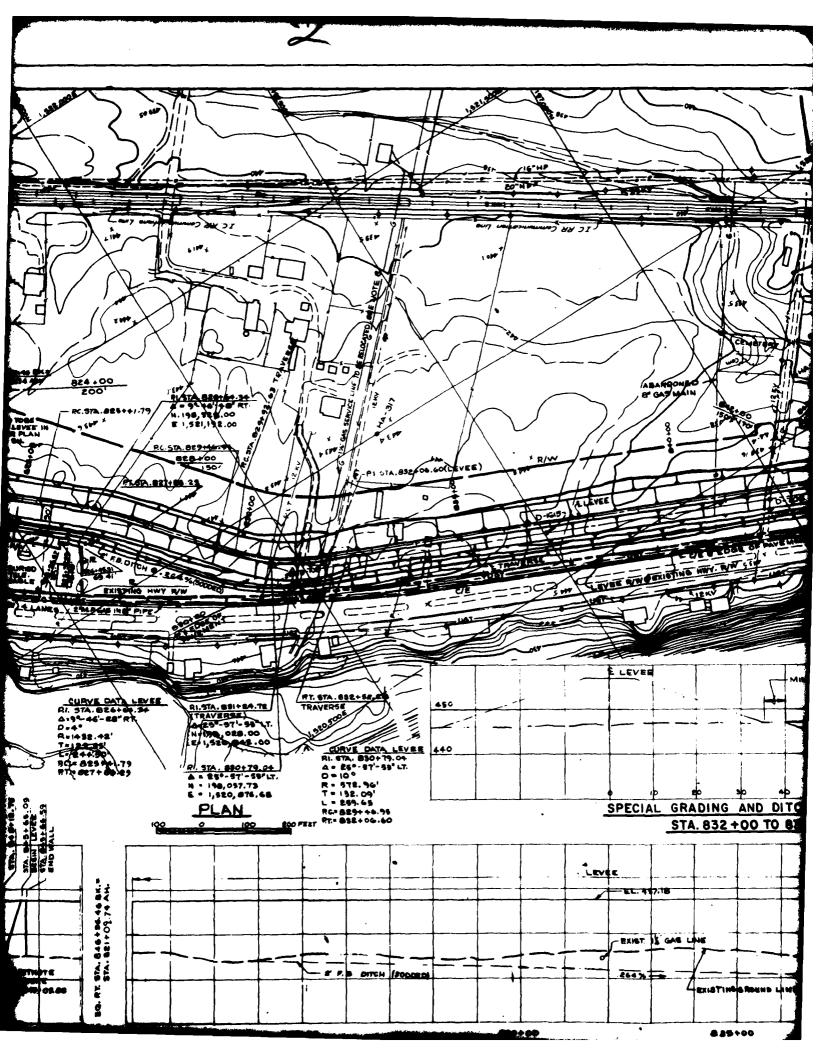


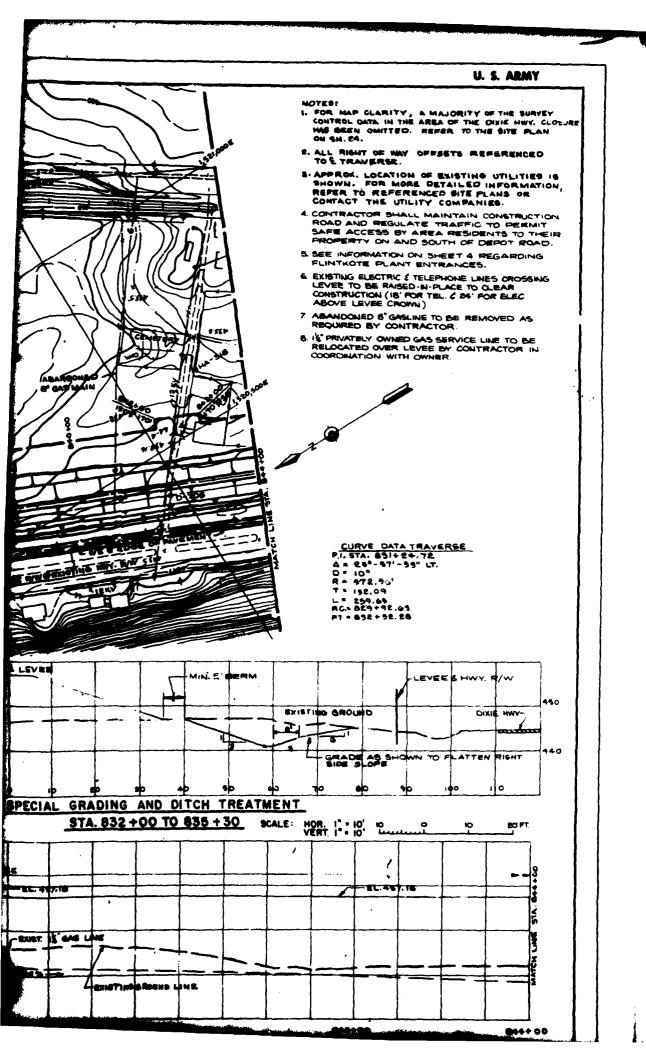


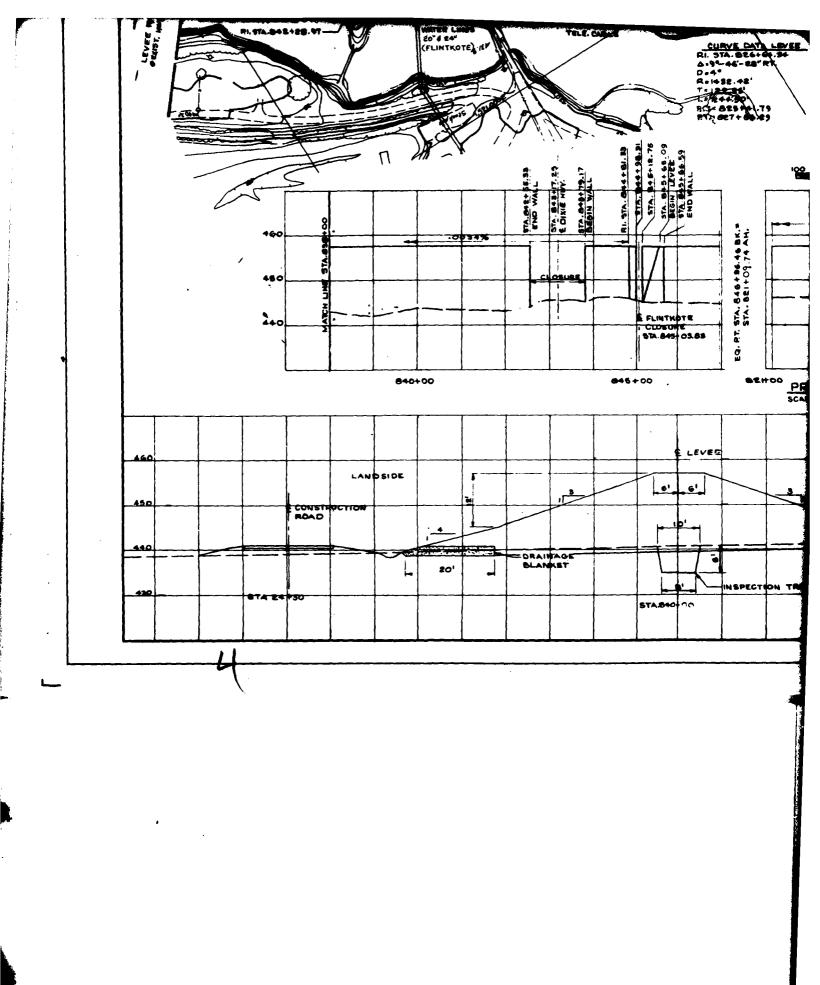


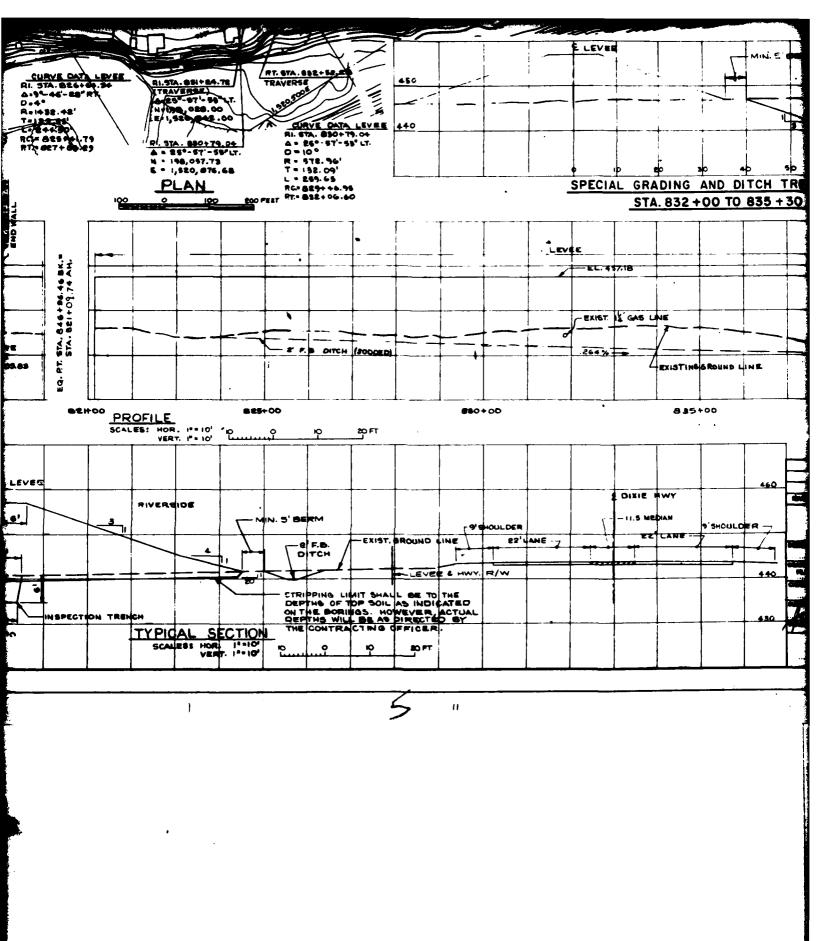


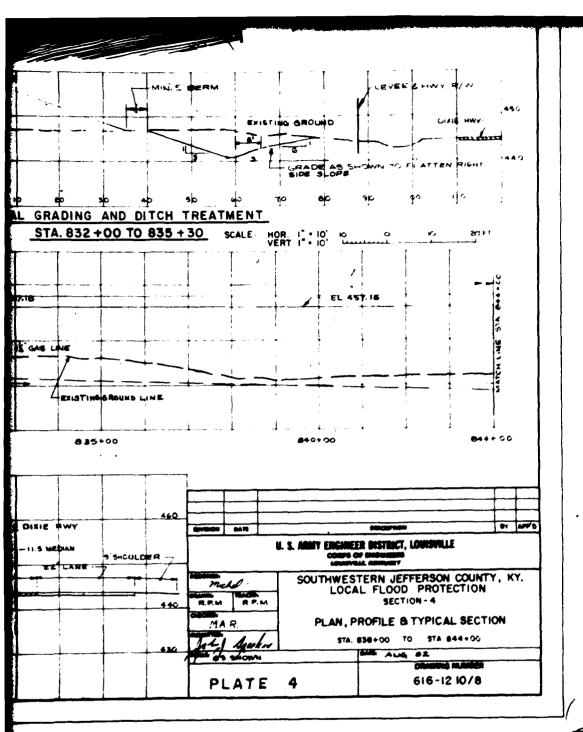


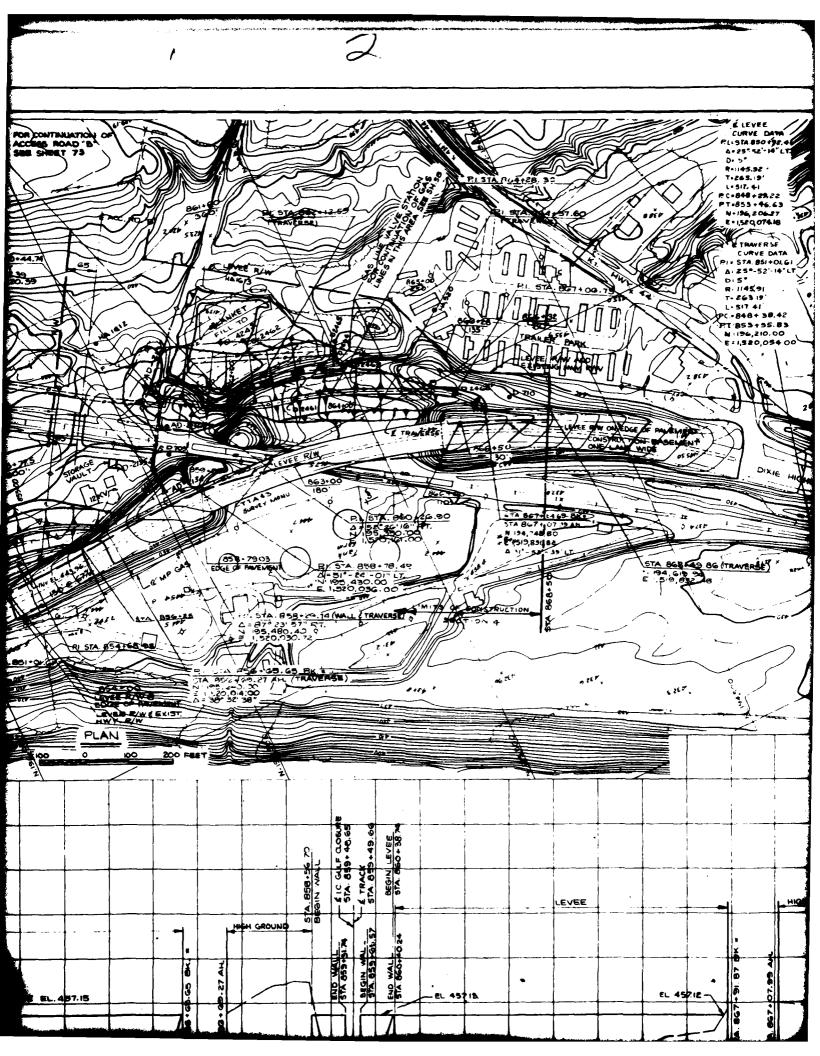


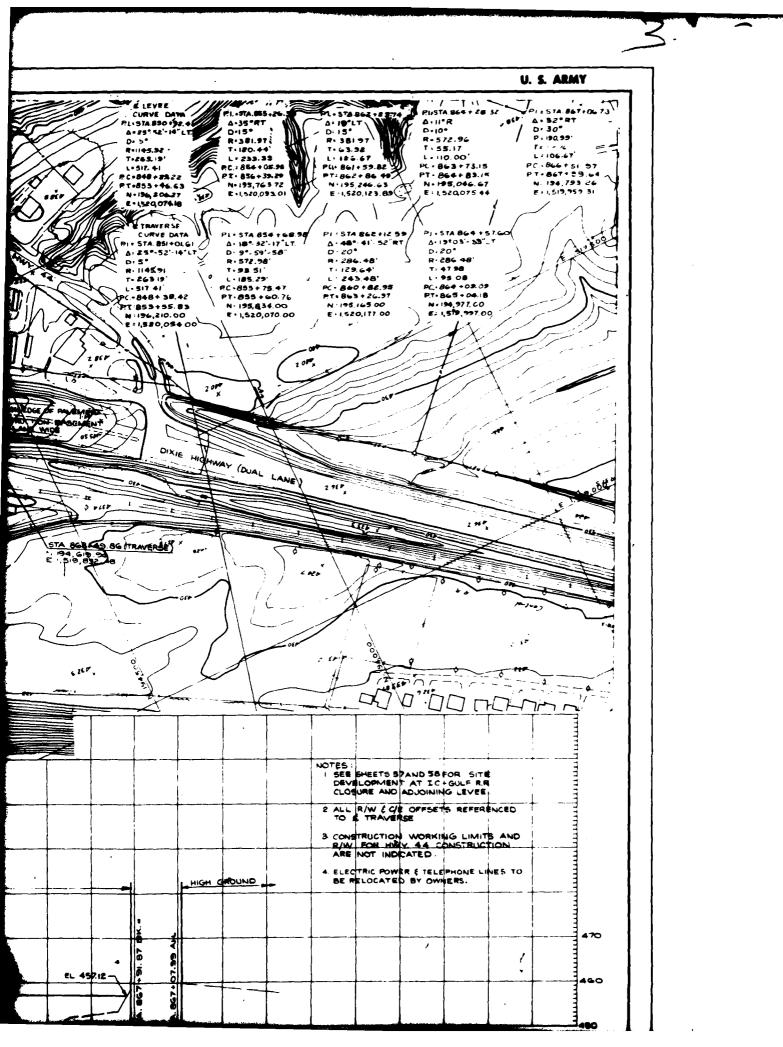


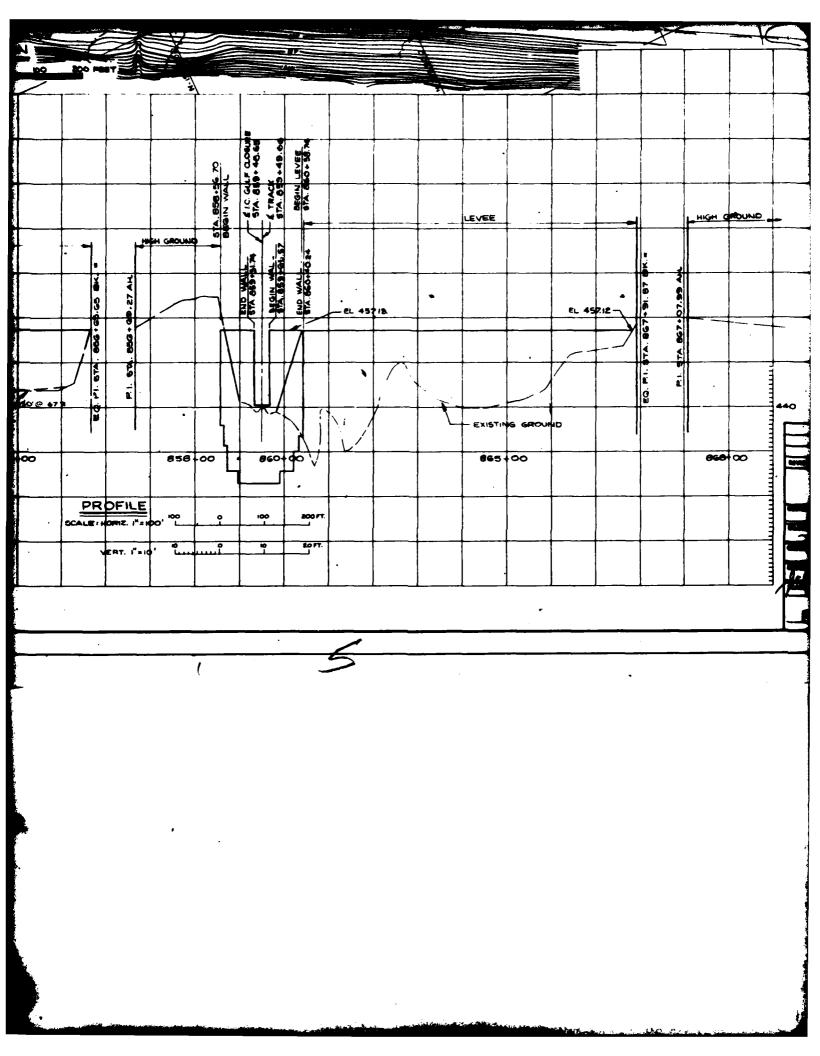


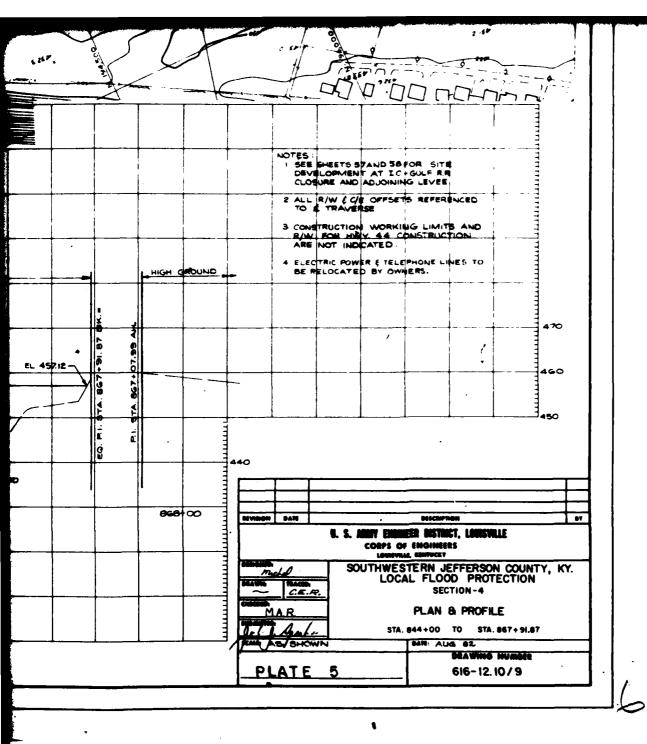


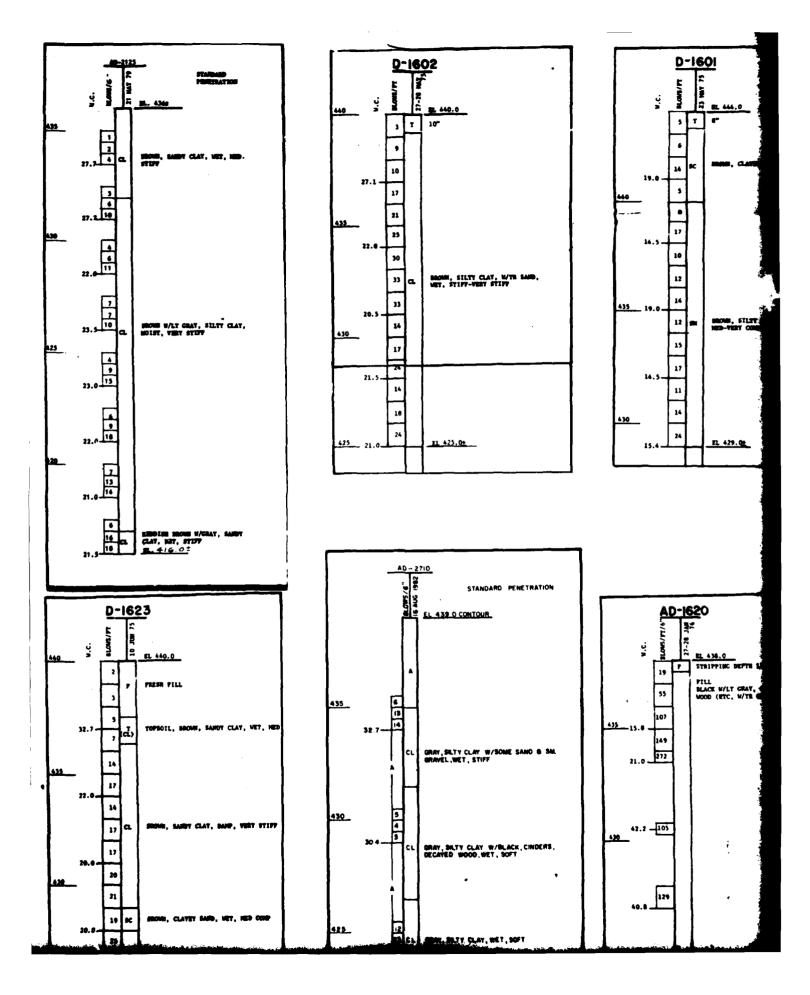


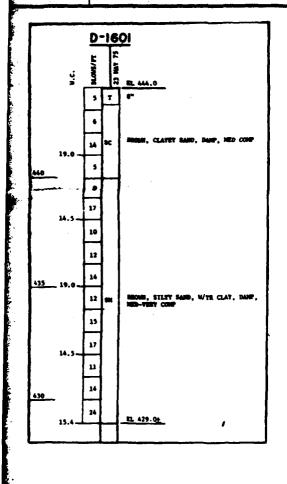






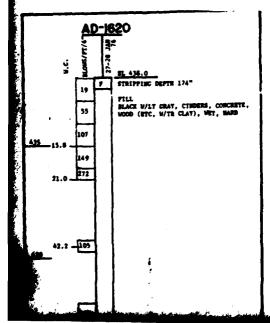


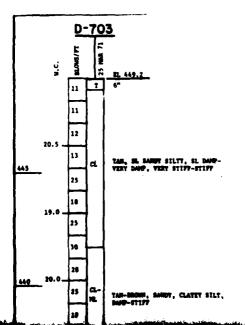




ž.	D 11/SHOTE	-16 sv vv 22	EL 439.0
1	3	1	10"
	,		NROWN, SILTY CLAY, DAMP, STIFF-
	10	CL.	BROWN, SILTY CLAY, DAMP, STIFF- VERY STIFF
18.1 ⊷	21	Н	
435	22		
}	_		
19.0-	29	CL	BROWN, SANDY CLAY, DANP, STIFF
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	20		
430 21 0	16		
430 21.0	19		
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425	17	-	
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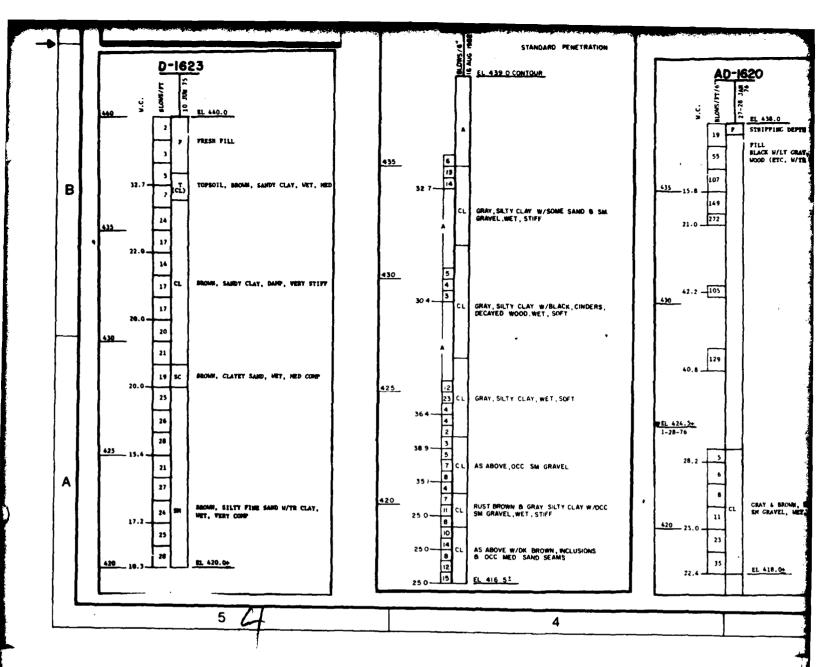
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ي ا	3		크 유 服 442.0				
16.7	Γ	Ŧ	STRIPPING DEPTH 312"				
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430 22.0-	<b>,</b>	1					
1	$\vdash$	<b>∤</b> ,	FILL MATERIAL, CINDERS, SOME GRAVEL, MOOD, BRICK, ETC., DAMP-WET, LOOSE				
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27.1 —	١,						
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EL 416.0+	,	L					
1-30-76 32.2	19		NY CAN'T LICENSES				
	32	СL	DK CRAY W/BROWN, SANDY CLAY, W/LG ROCK, MET, SOFT				
24.0 —	32						

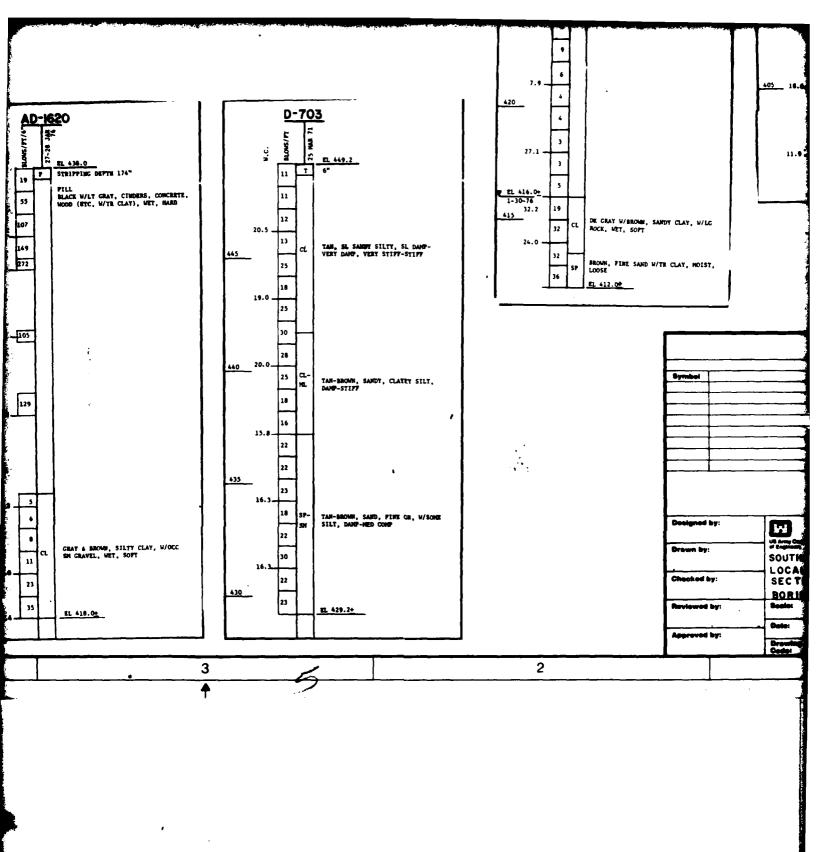


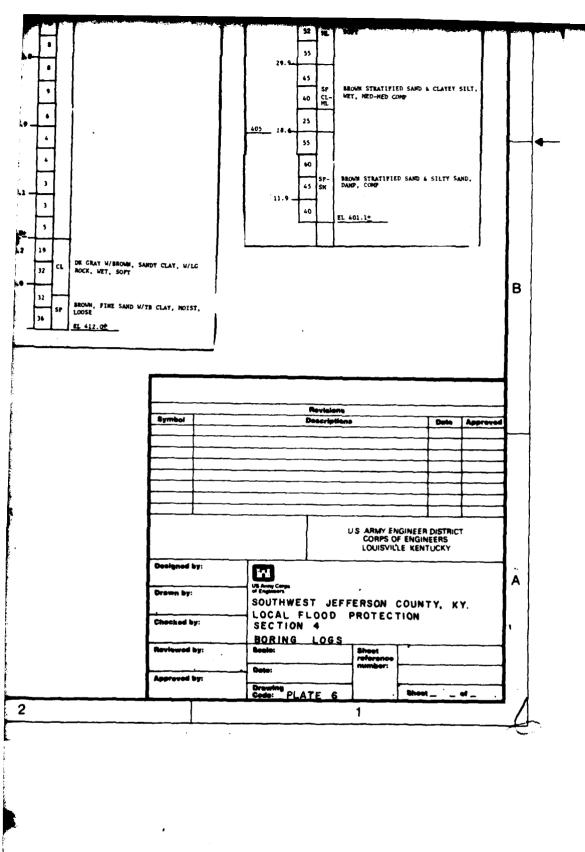


SP SROWN, FIRE SAND W/TR CLAY, MOIST,

2 1 D-704 D-1620A EL 442.0 EL 426.1 F STRIPPING DEPTH 312" • 10 BLACKISH CRAY, SL OBGANIC, SANDY SILTY CLAY, DAMP, NED 34 BROWN, SILTY CLAY, DAMP, STIFF-VERY STIFF 440 D 20 7 35.1 11 8 MOTTLED GRAYISH TAN, SAMBY SILTY CLAY W/TR IBONOXIDE, SL WET, MED . SAMBY CLAY, DAMP, STIFF . 32.2 420 12 GRAY TAN, SILTY SANDY CLAY, YERY DAMP, STIFF CL 6 18 7 21 EL 416.1+ 3-29-71 REDDISH TAM, SILTY CLAY & SAND W/TR IRONOXIDE, DAMP-HOIST, MED-COMP 20 415 28 430 22.0 21.0 5 35 FILL MATERIAL, CINDERS, SOME GRAVEL, WOOD, BRICK, ETC., DAMP-WET, LOOSE DK TAN BROWN, SILTY SANDY CLAY W/TR IRONOXIDE PELLETS, DAMP, STIFF 28 12 C EL 424.0+ 16.3 23.0 7 25 410 TAN BROWN, SANDY CLAYEY SILT, WET, SOFT 8 52 425 . 55 BROWN STRATIFIED SAND & CLAYEY SILT, WET, NED-HED COMP 40 55 420 BROWN STRATIFIED SAND & SILTY SAND, DAMP, COMP 45 27.1 11.9 ) 40 EL 401.1+ EL 416.0+ 413 32.2 19 DE CRAY W/BROWN, SAWDY CLAY, W/LG ROCK, WET, SOFT 32 В SL SAMOT STLTY, SL DAMP-OF DAMP, YERY STIFF-STIFF SROWN, PIRE SAND W/TR CLAY, HOIST, LOOSE 36







	D-1	61	5
ن غ 27.۶	TH /SWS/FT	3 FFB 75	EL 443.0
440 24.5 -	7		
18.1 —	35 42		DK SROWN, SILTY CLAY, WET-DAMP, MED-STIFF
435	37 C1		MED-STIFF
20.0 —	39 74		•
22.4 —	56 88		
17.2		ıc	BROWN, CLAYEY SAMB, MET, COMP
425 17.6	42		
8.7_	••	SDR	BROWN, SILTY FINE SAMD, DAMP, COMP EL 423.04

D-706					
<b>H</b> .C.	NLOWS/FT	26 MAR 71	EL 443.1		
	6	F	6"		
,	9	C1	TAN GRAY, SANDY SILTY CLAY W/SL ORGANIC FILL MATL, SL DAMP, SYIFF		
440 20.1-	<del> </del>	┡			
	11				
	23	CL	DK GRAY TAN, SL SANDY SILTY CLAY, DAMP, STIFF, W/TR ORGANIC FILL MATL		
19.5	29				
	40				
	43	a	DK REDDISH BROWN, SL SANDY, SILTY CLAY, W/SPOTTED IRONOXIDE, DAMP,		
435	49	1	STIFF-VERY STIFF		
19.0 _	60				
	16	cı.	MOTTLED GRAY REDDISH BROWN, SL SANDY, SILTY CLAY, VERY DAMP, STIFF-MED		
18.1 _	18	L	STIFF-NED		
430	20				
-	26		MOTTLED GRAY DK BROWN, SILTY CLAY W/TR IRONOXIDE, LT SAND, DAMP, VERY		
	24	CL	W/TR IRONOXIDE, LT SAND, DAMP, VERY STIFF		
21.5_	32				
	17				
425 20.0 -	,	CL- MI, SM	BROWN, VERY SANDY CLAYEY SILT, SL WET, MED-COMP		
	19				
	29	_]	EL 423.1+		
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D-1618							
	BLOWS/FT	2 PEB 75	EL 444.0				
37.6	9	1	6"				
	24						
	20						
20.0	11						
440	25						
]	ļ.,						
18.6	34		į				
	19						
1	36						
1	43	a	BROWN, SILTY CLAY				
<u>10.1 – </u>	31		DAMP, STIFF-VERY				
1	61						
	95	•					
21.5-	23						
430	48						
21.0-	62						
	33						
	56						
20.5	59						
425	36	-	1				
	55	cı	BROWN, SANDY CLASS				
18.6	<b>.</b>	1	EL 424.04				

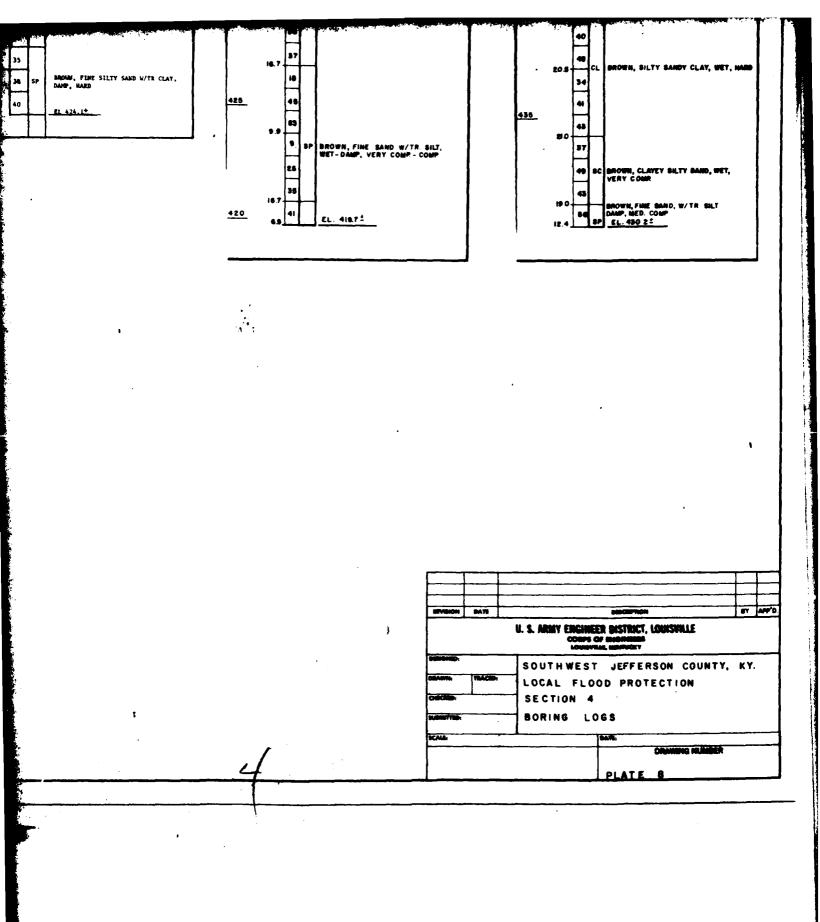
SILTY	W/FINE	SAMP,

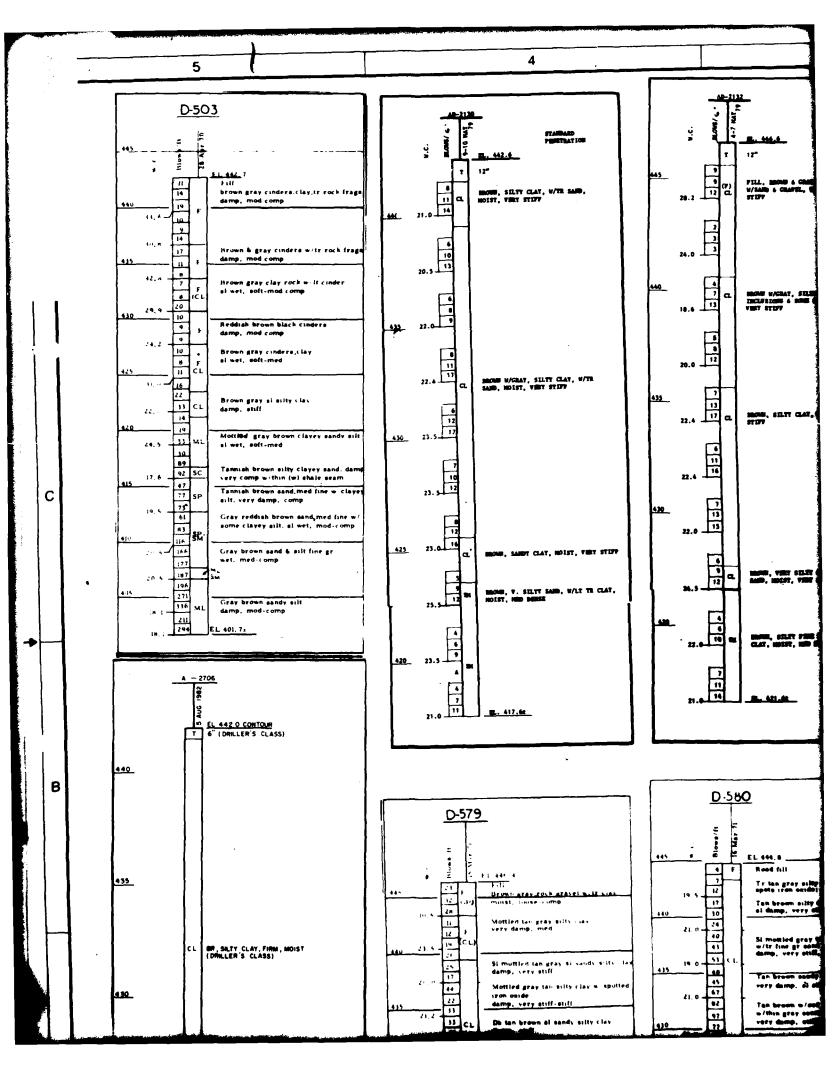
	D.	-70	08
W.C.	BLOWS/FT	10 JUL 71	USED 24" x 5" SAMPLER
	,	7	12"
20.0-	21		
440	41		
	49	ա	REDDISH BROWN & GRAY, MOTTLED, CLAY, SL SILTY, SL DAMP, FIRM
19.5-	61		
1	13		
ł	24		
435 21.2-	22		
[	19	СL	BROWN, LEAN CLAY W/TR SAND, SL DAMP, FIRM
İ	24		
21.2-	29		
	35		
430	36	a.	BROWN, SANDY SILTY CLAY, SL DAMP, HARD
21.5	38		-
1	33		
	34	L_	
15.8	35		
425	38	SP	BROWN, FINE SILTY SAND W/TR CLAY, DAMP, HARD
	40		EL 424.1*
}			

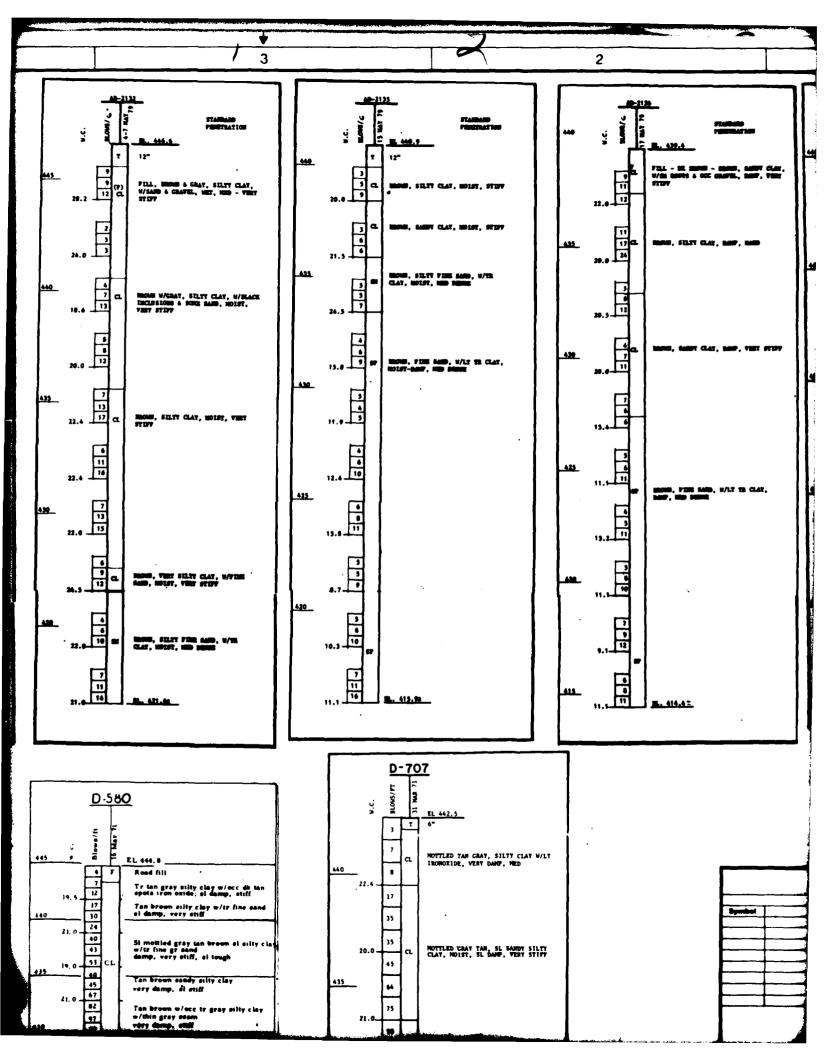
	D-2461 L 0 0					
İ	DB L/SMO/IB LL: 444.7					
	1					
	17					
į	24					
}	23.5 CL BROWN, SILTY CLAY, W/TR. SAND					
!						
440	50					
	10.6					
1	95					
]	108					
İ	93					
435	19.5					
	13 CL BROWN, VERY SAMDY SILTY CLAY,					
	WET, COMP.					
İ	21.0					
	20					
	36					
430	14.5 SM BROWN, SILTY FINE SAND, W/TR					
1	CLAY, WET, VERY COMP					
	36					
1	37					
	16.7					
105	H I					
425	45					
	9.9 83					
}	9. SP BROWN, FINE SAND W/TR. SILT, WET-DAMP, VERY COMP-COMP					
1	25					
	35					
420	16.7 41 EL. 419.7					
	6.8 <u>EL. 419.7 -</u>					

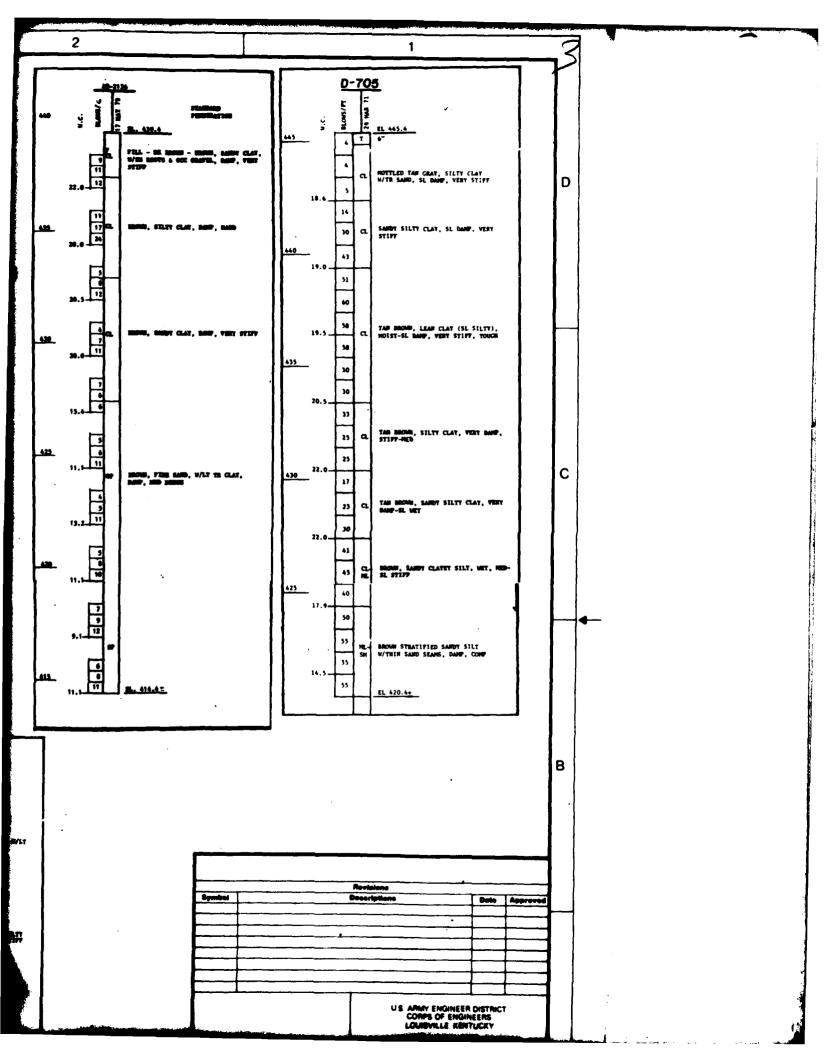
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	-	0-24 ET	981 <u> </u>
	Ö.	LOWS/	161 8 6 6 6 6 6 6 8
	•	ľ	EL. 444.7
	17	1	
	24	] 	BROWN, SILTY CLAY, W/TR. SAND
•	24		WET, HARD
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10.0	77		
	96		
	105		
ia.	93	ļ.	
فمخصه	13	a	ORANG MEDICAL AND AND AND AND AND AND AND AND AND AND
	23	"	BROWN, VERY SANDY SILTY CLAY, WET, COMP.
21.0	29		
	36		
	46		
14.5	5.2	384	BROWN, SELTY FINE SAND, W/TR. CLAY, WET, VERY COMP.
	36		
<b>10</b> 7	37		
	10		
	45		
•.•	83		
	•	5P	BROWN, FINE SAND W/TR. SILT, WET-DAMP, VERY COMP COMP.
	25		
16.7	35	4	
6.0	41	لـ	EL. 419.7 4

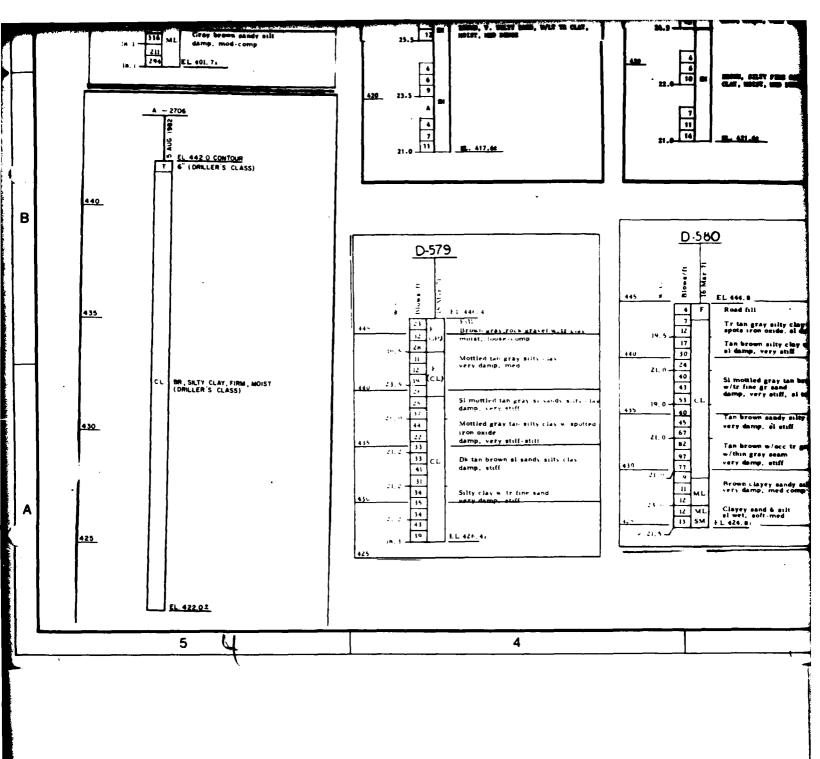
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}		26	•	}
ì	141.	15	a	SROWN, SANDY CLAY, W/SAND SEAMS & OCC. ROCK (LG. L.S.)
}			l	DAMP, HARD
450		25	l	}
	15.4-	31		
ł		24		
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i		35		
ł	14.5-	21		
445		35		
ĺ		<del> </del>	1	
į	14.1 -	36		
•		19		
[		39	cı	BROWN W/RED. BROWN & OCC. BRAY
<b></b>	•••	51		BROWN W/RED, BROWN & OCC. GRAY LEAN CLAY W/DARK BROWN INCLUSIONS DAMP-WET, HARD
440	20,5	28		
ĺ		40		
l		43		
	20.5	-	CL	BROWN, SILTY SANDY CLAY, WET, HARD
	ļ	34		
435		41		
{	<b>8</b> .0.	43	Ĺ	
· ·		37		
	ı	49	8¢	BROWN, CLAYEY BILTY SAND, WET, VERY COMP
}		43		Tuni ywmi
}	19.0-	56		BROWN, FINE SAND, W/TR. SILT DAMP, MED. COMP.
ł	12.4	لــا	<b>8</b> P	EL. 480.2 =

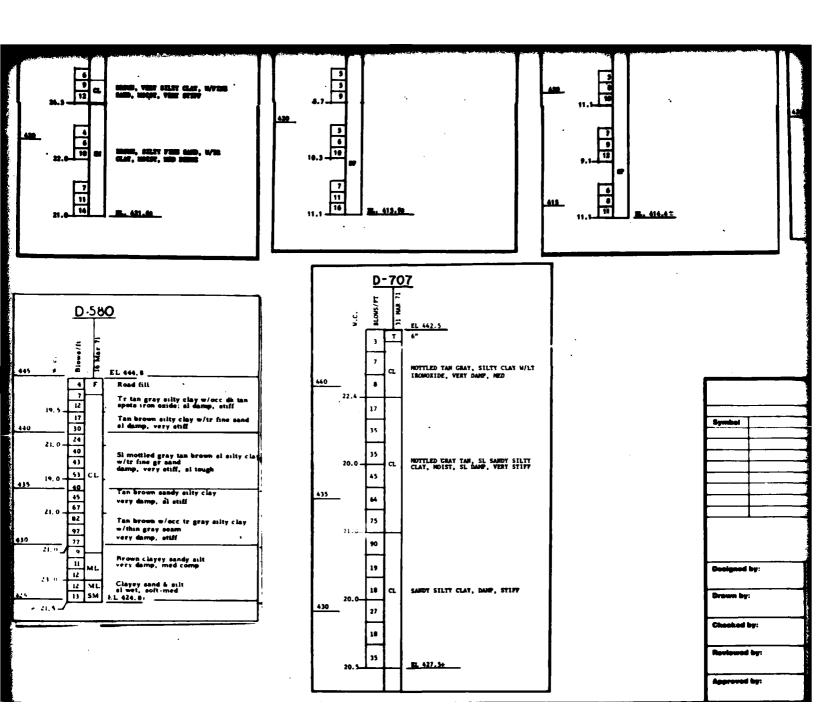


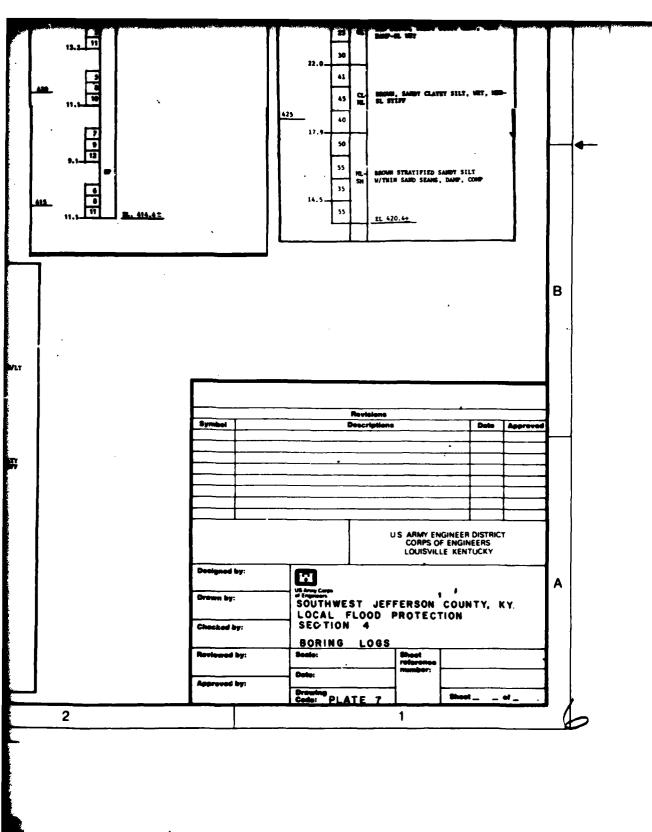


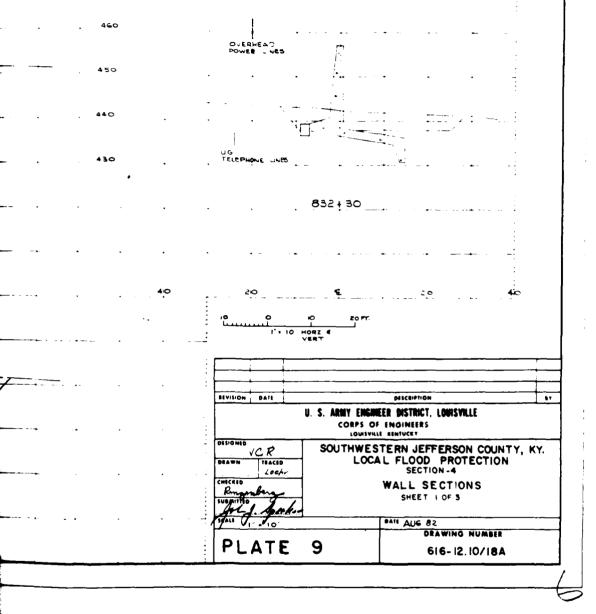






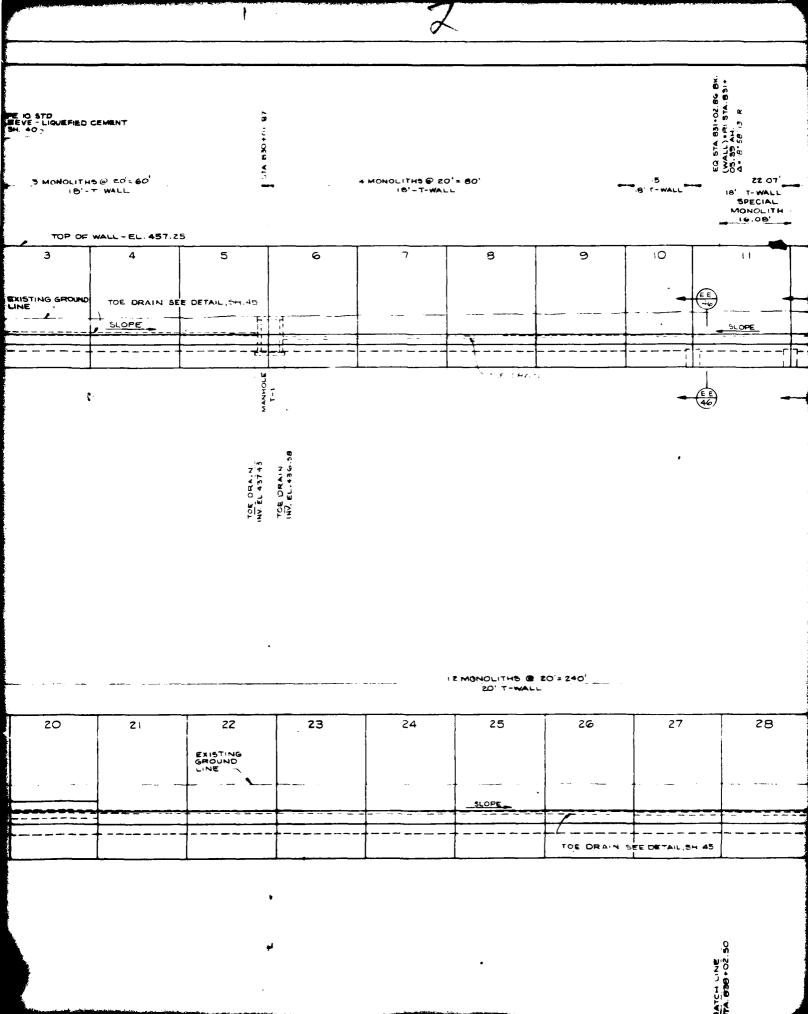




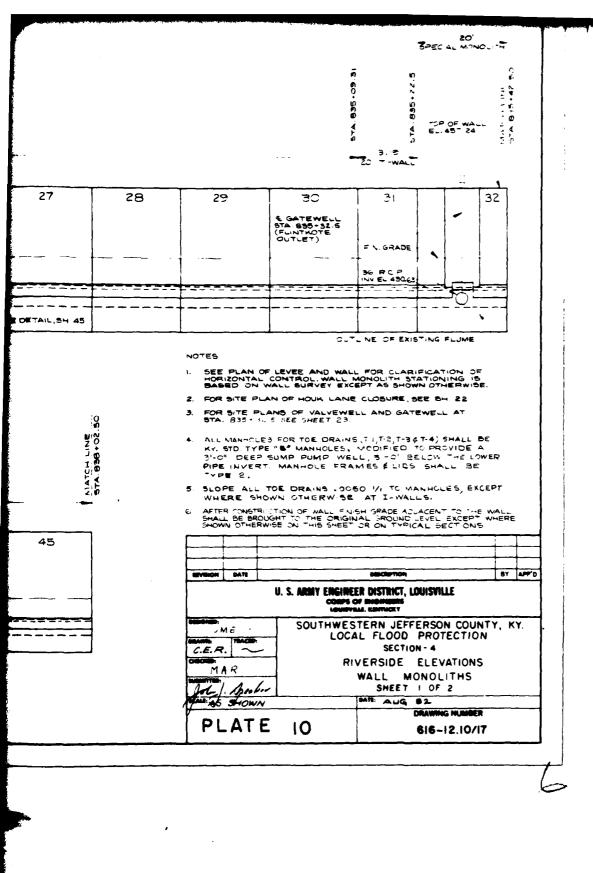


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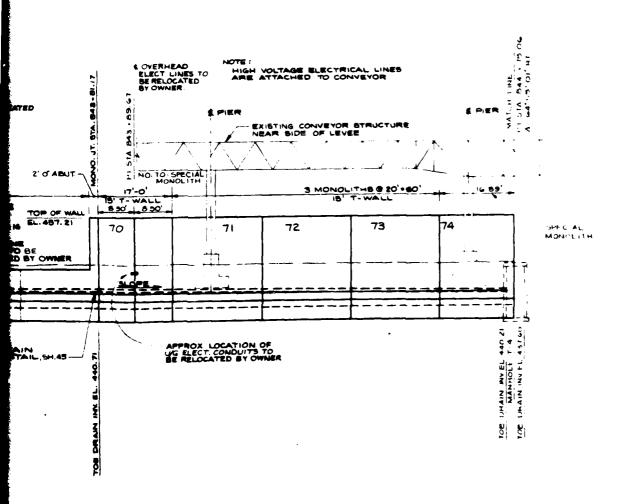
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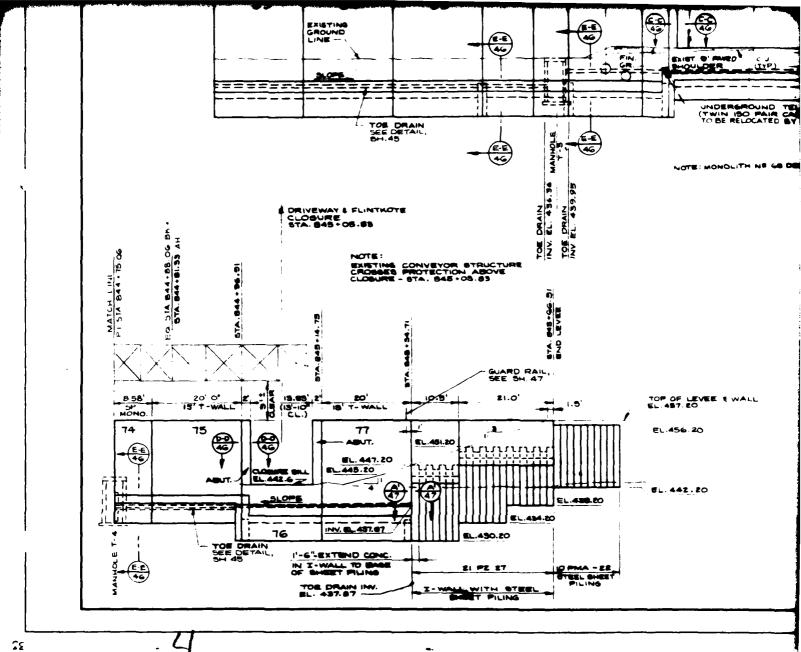


0 2	EXISTING GROUND LINE	23	24	25	26	27	28	
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						AIATOL LOTAIN	66 .⊀	
	11 MONOLITHS @ 20'-	220'						
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	EXISTING GROUND LINE							
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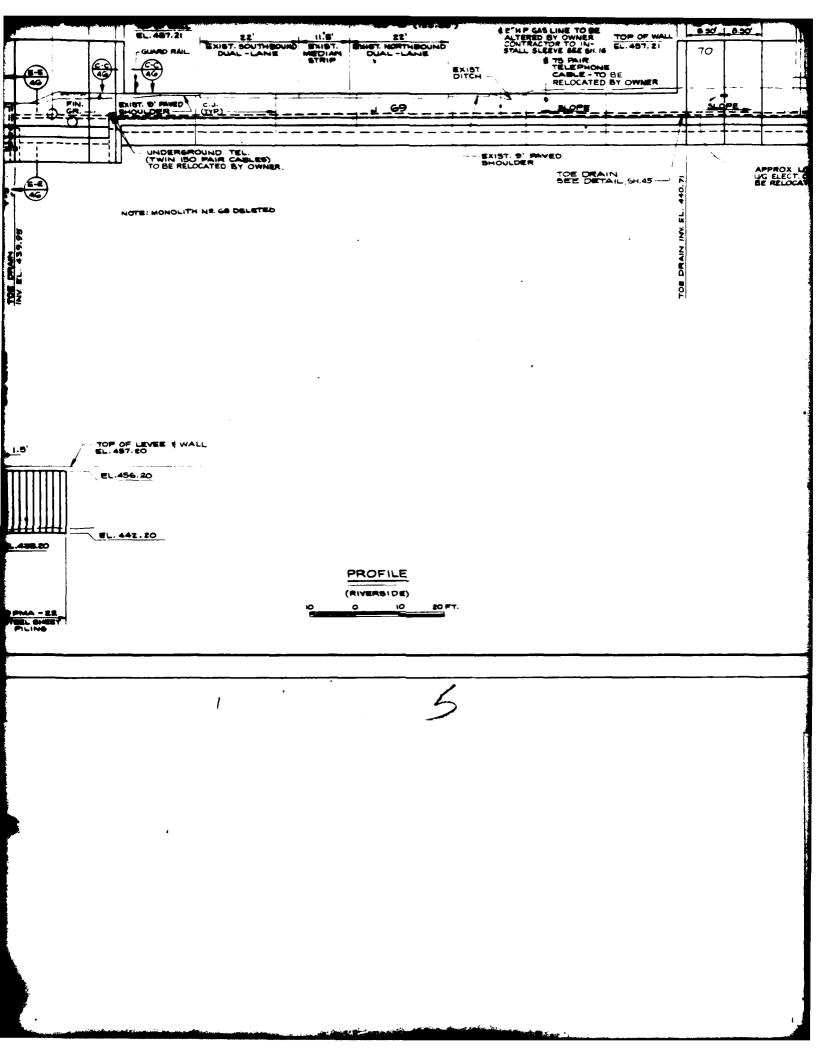


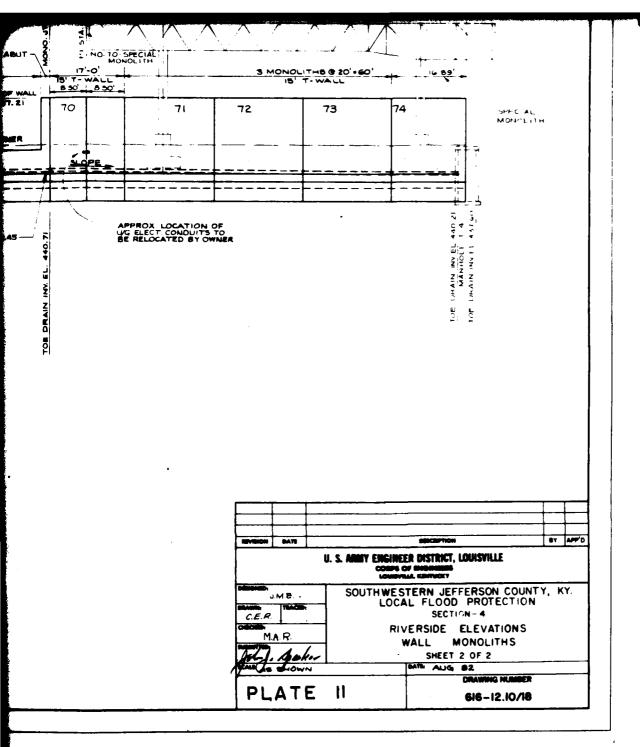
<u> </u>							
			87A. <b>940 + 02.8</b> 0		MATCH LINE 07A:041-042:80		
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57	56	59	60	61	62		
		•					
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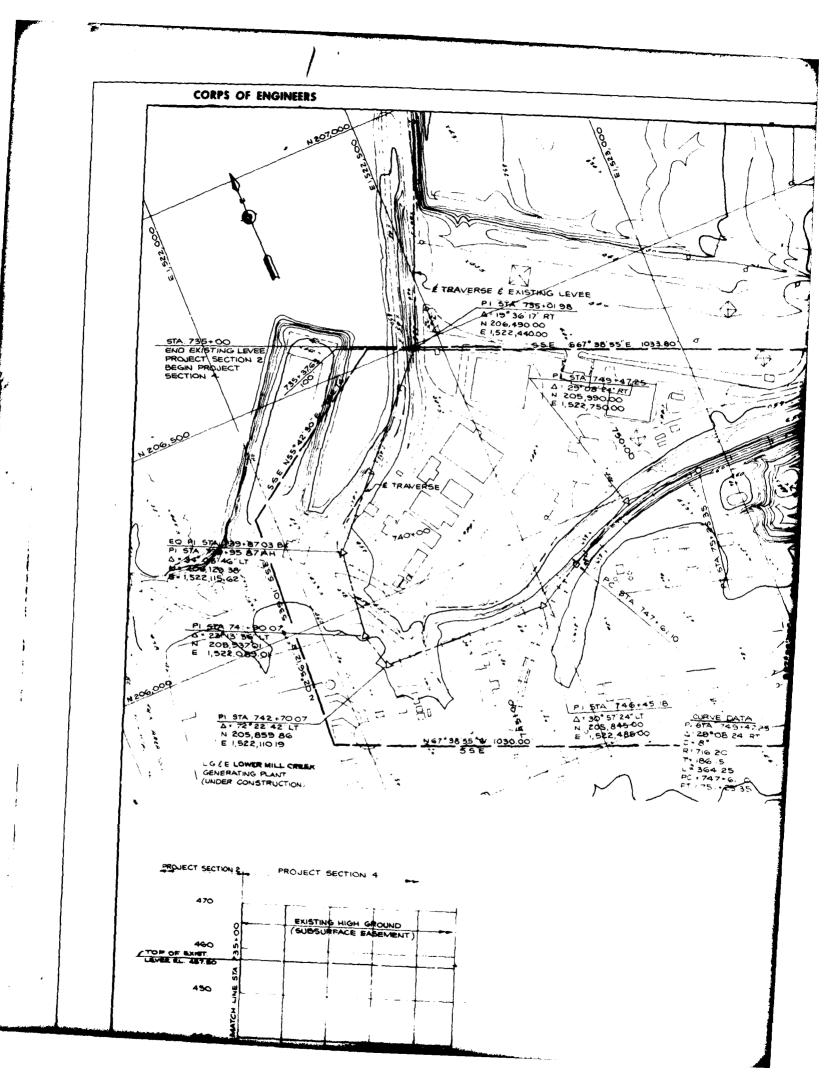


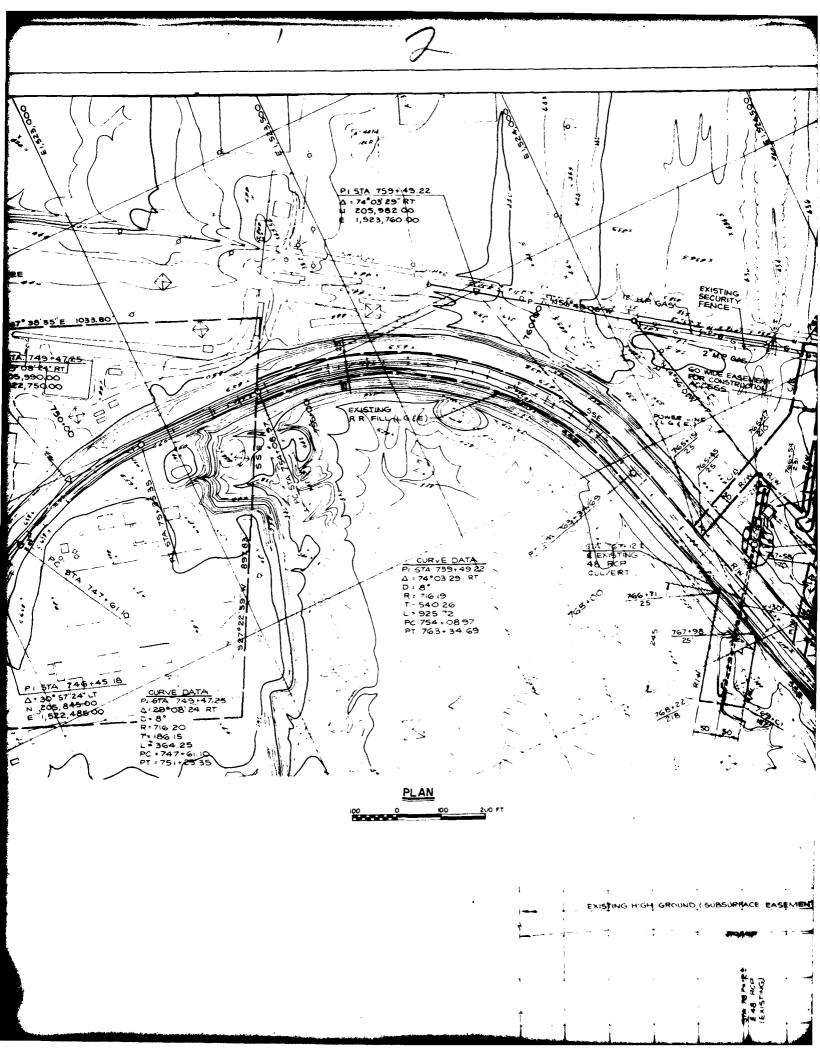
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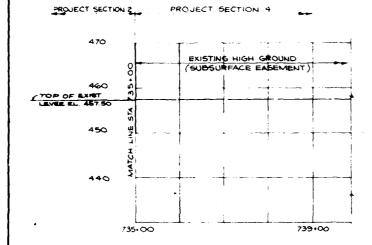




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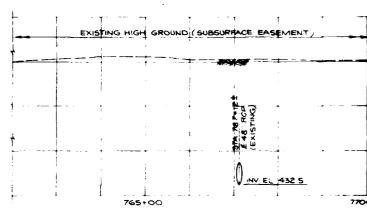


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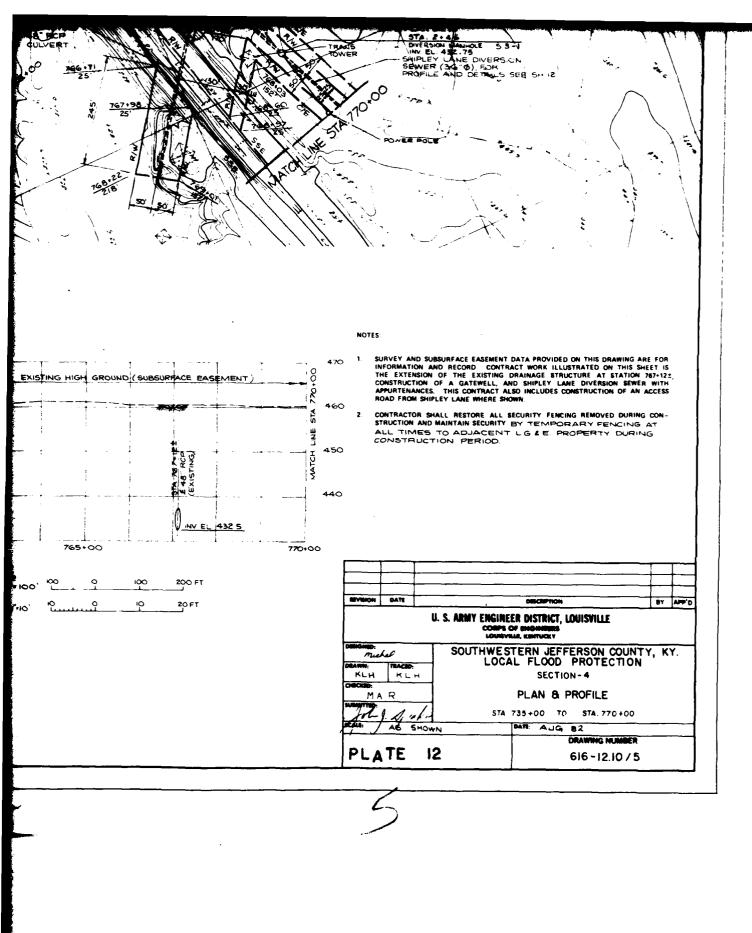


PROFILE

SCALE: HORIZ 1"= 100" 100 200 FT

VERTICAL 1"=10" 100 0 10 20 FT

(





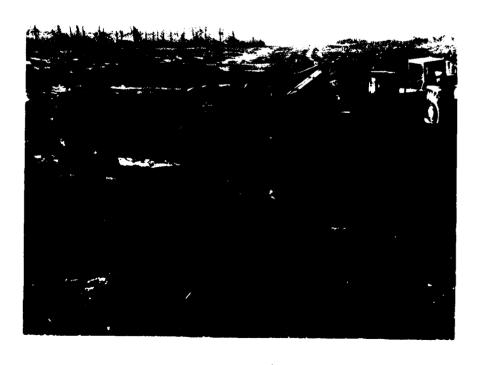
STA 775+50 ±



STA 776+00 ±



STA 777+00



STA 777+00 -



STA 777+00 -



STA 777+30



STA 777+00



STA 778+00



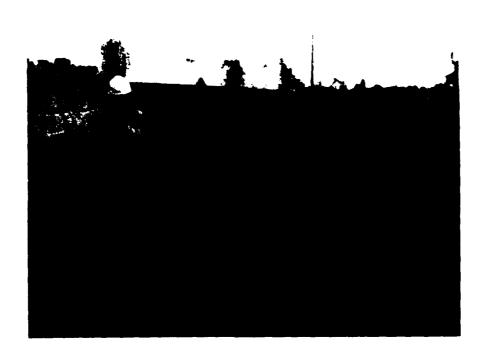
STA 778+00



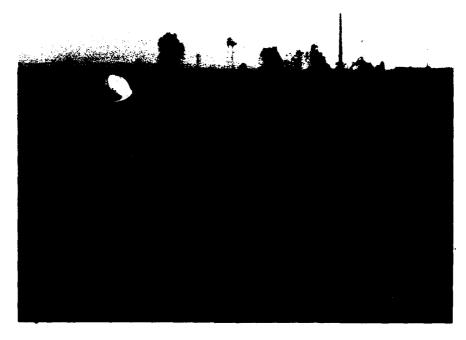
STA 779+00



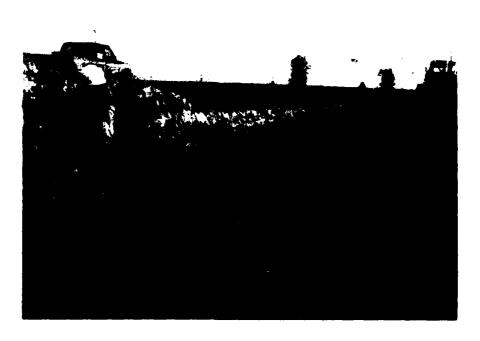
STA 780+00



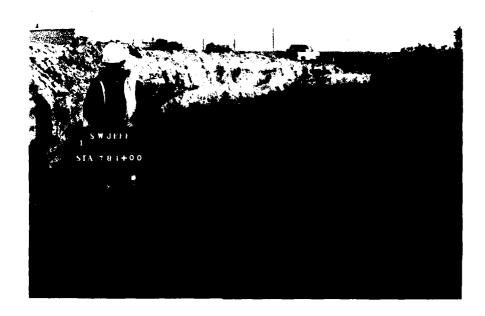
STA 781+JO



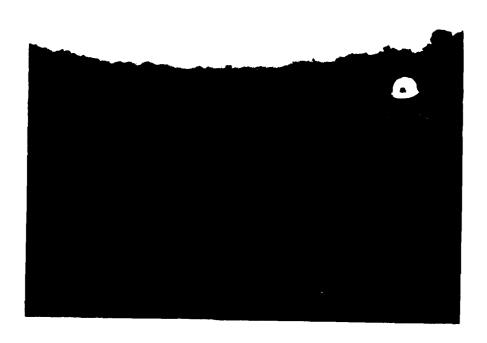
STA 782+00



STA 783+00



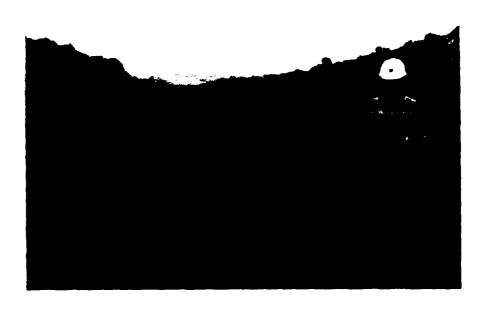
S1A 784+00



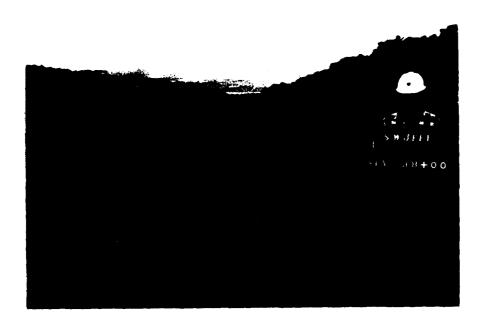
STA 785+00



STA 786+00



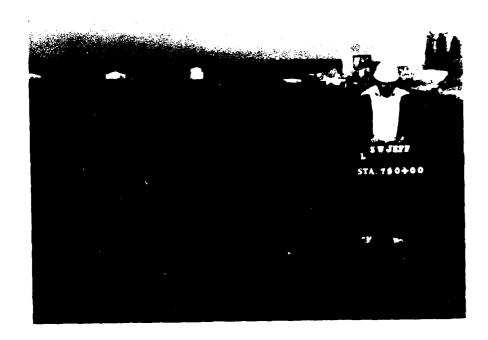
STA 787+00



STA 788+00



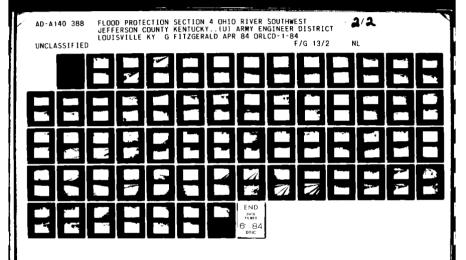
STA 789+00

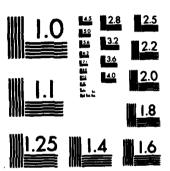


STA 790+00

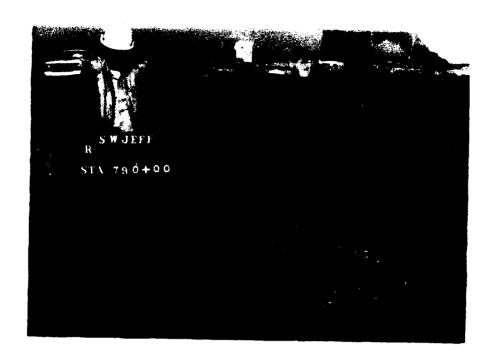


STA 790+00

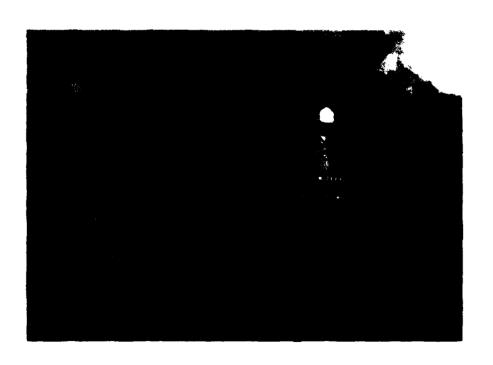




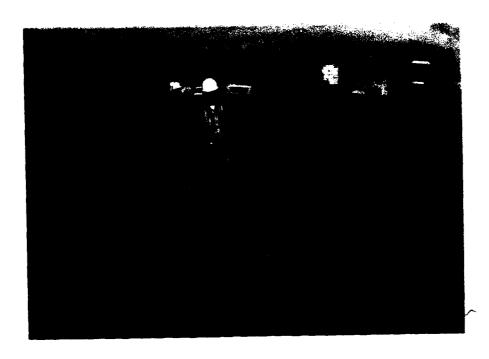
MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS-1963-A



STA 790+00



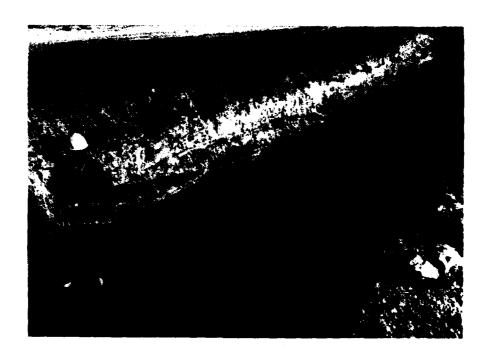
STA 791+00



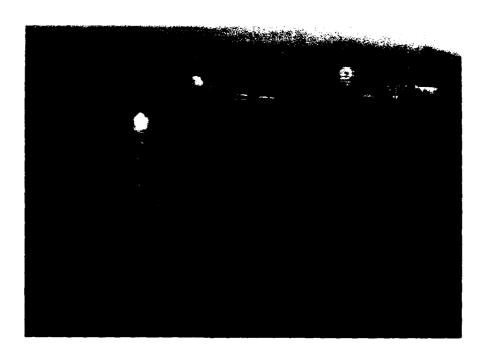
STA 791+00



STA 791+00



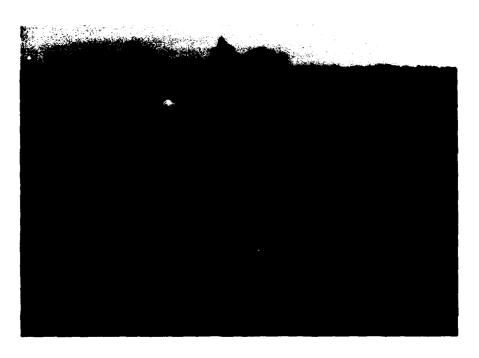
STA 792+00



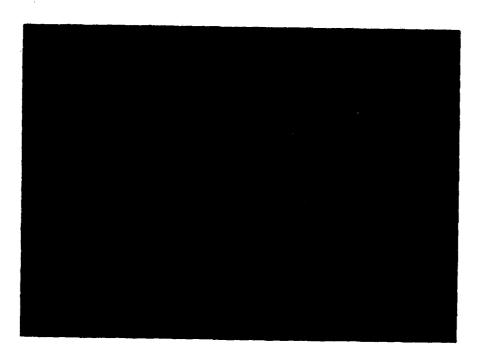
STA 792+00



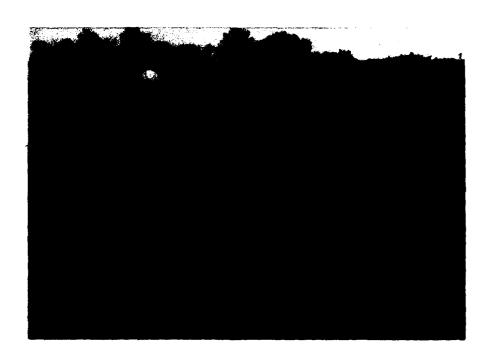
STA 794+00



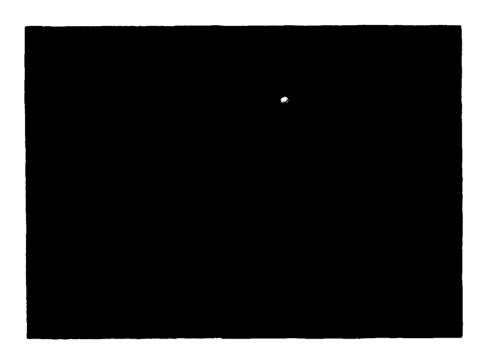
STA 794+00



STA 795+00



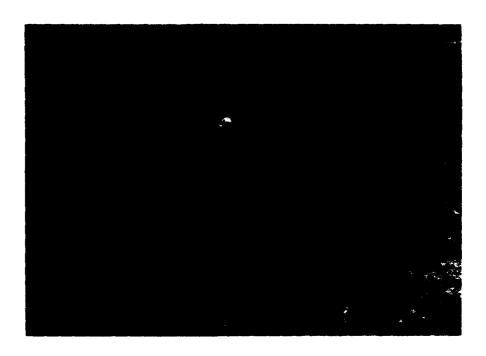
STA 795+00



STA 796+00

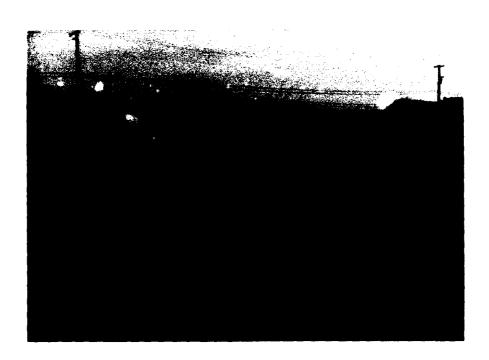


STA 796+00

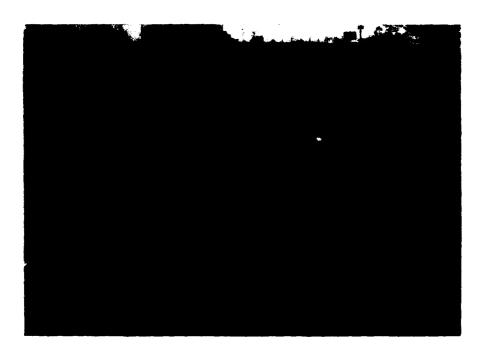


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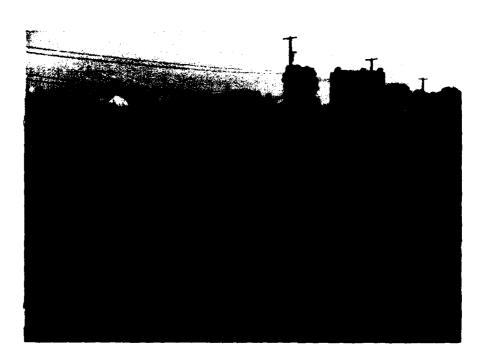
STA 797+00



STA 797+00



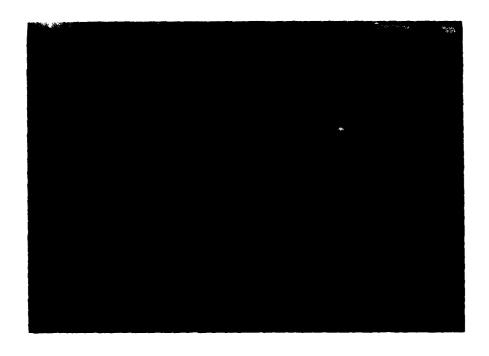
STA 798+00



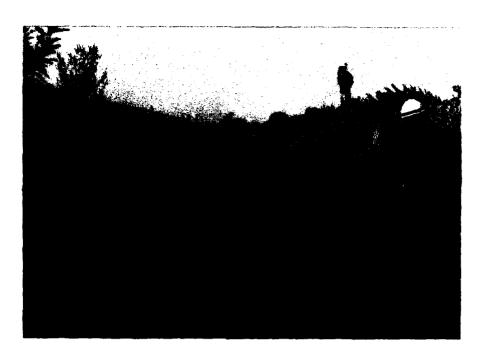
STA 798+00



STA 799+00



STA 810+00



STA 811+00



STA 811+00



STA 811+50



STA 812+00



STA 812+00



STA 813+00



STA 813+00



STA 814+00



STA 814+00



STA 815+00



STA 815+00



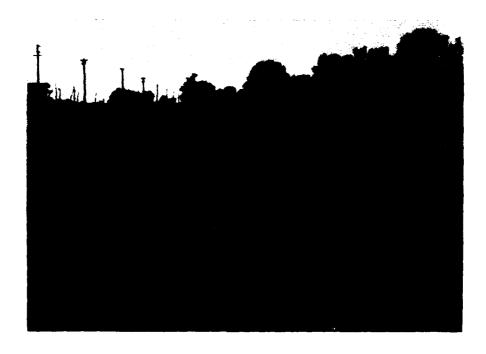
STA 816+0U



STA 816+00



STA 817-00



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STA 817+00 -



STA 817+00



STA 818+00



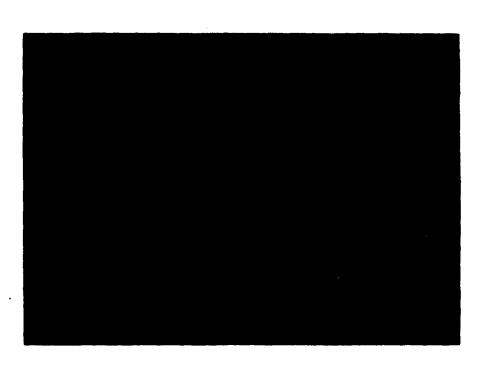
STA 818+00



STA 819+00



STA 819+00



STA 819+00



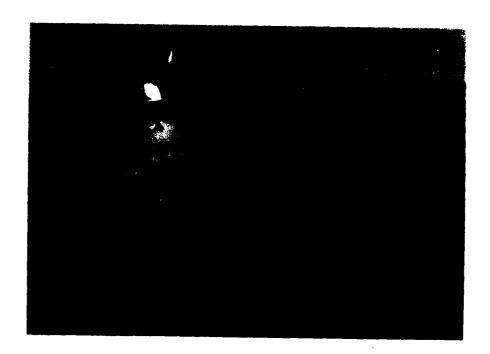
STA 820+00



STA 820+00



STA 820+00



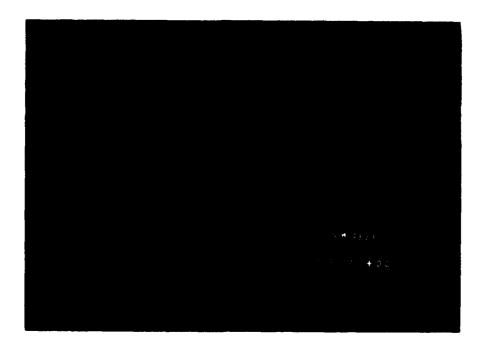
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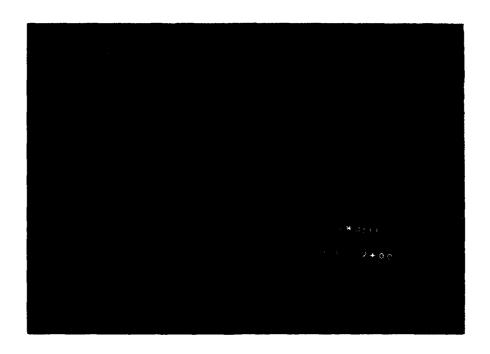
STA 821+00



STA 821+00



STA 821+00



STA 822+00



STA 822+00



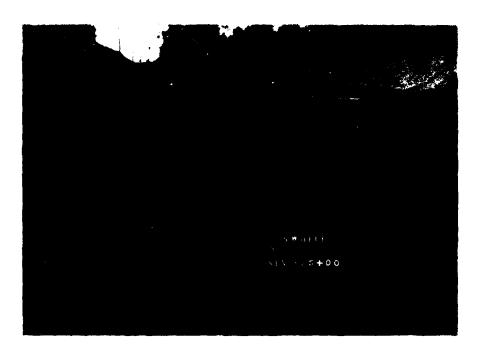
STA 822+00



STA 823+00



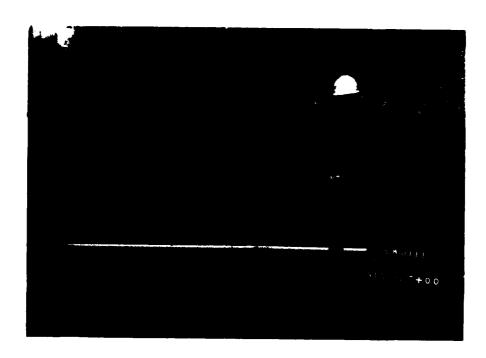
STA 823+00



STA 825+00



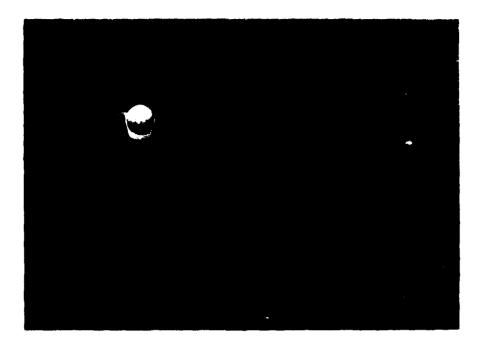
STA 826+00



STA 827+00



STA 828+00



STA 829+00



BACKFILL AT T-50



UNSUITABLE MATERIAL TO BE REMOVED (T-50)



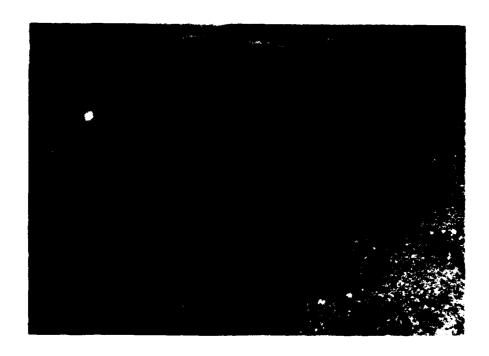
UNSUITABLE MATERIAL TO BE REMOVED (T-50)



EXCAVATION FOR WALL (T-50)



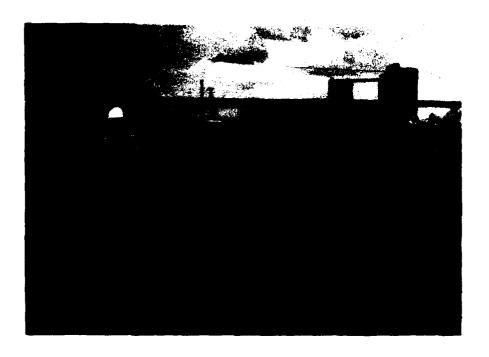
EXCAVATION FOR WALL (T-50)



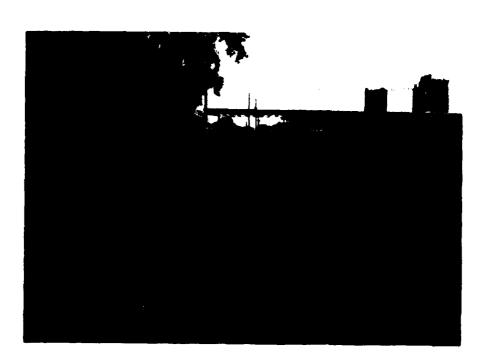
STA 823+00



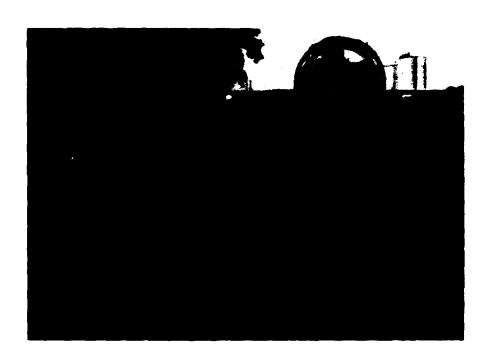
STA 824+00



STA 825+00



STA 826+00



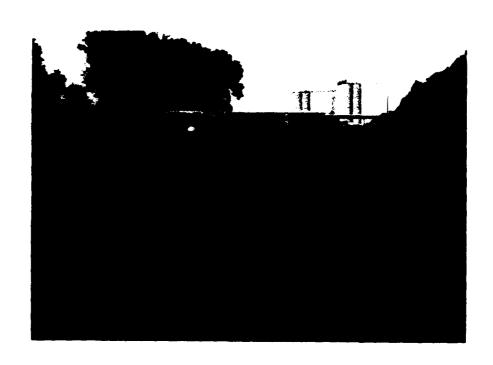
STA 827+00



STA 828+U0



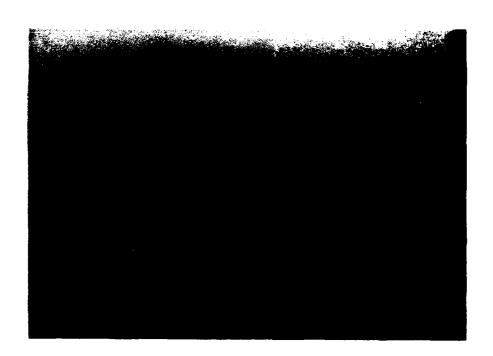
UNSUITABLE MATERIAL STA 828+50



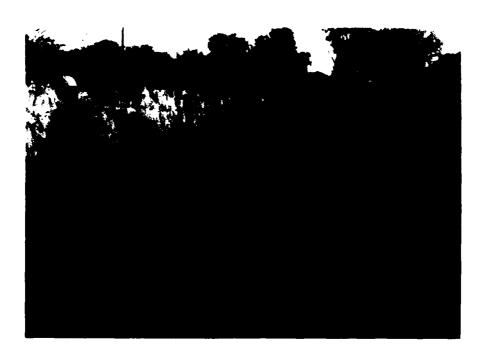
STA 829+00



STA 830+00



STA 830+00



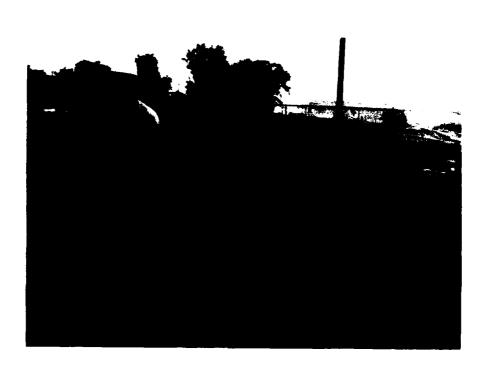
STA 831+00



STA 831+00



STA 832+00



STA 832+00

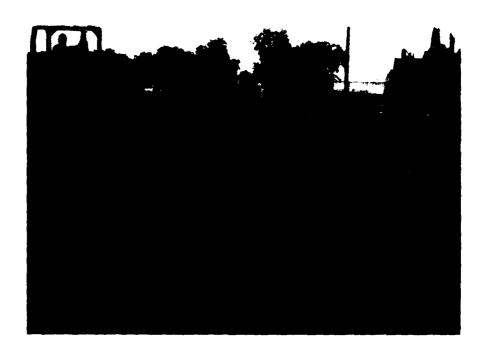
K



STA 832+00



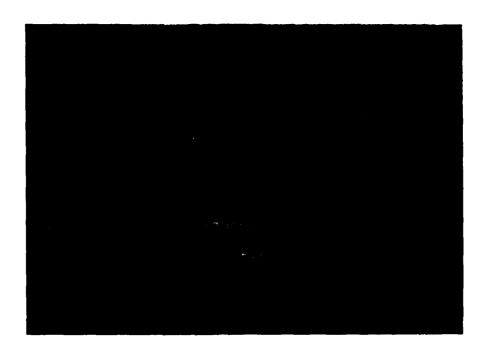
STA 833+00



STA 833+00



STA 833+00



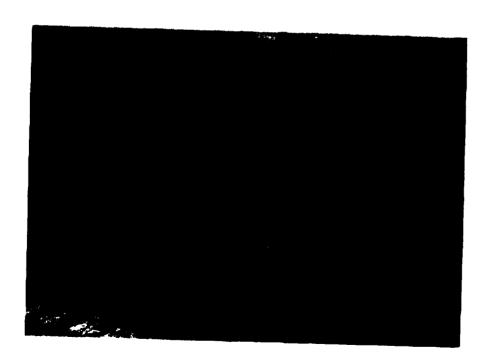
STA 834+00



STA 835+00



STA 835+00



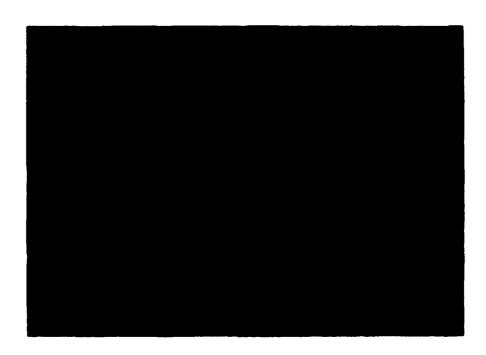
STA 836+00



STA 836+00



STA 837+00



STA 837+00



STA 838+00



STA 838+00



STA 839+00



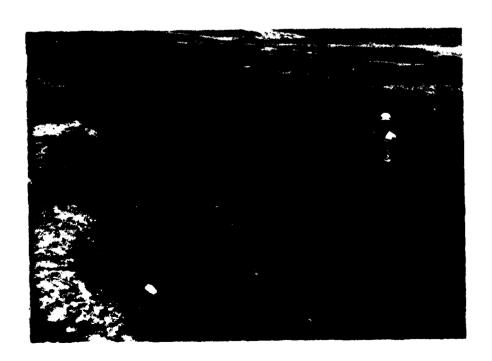
STA 840+00



STA 841+00



STA 841+00



STA 842+00



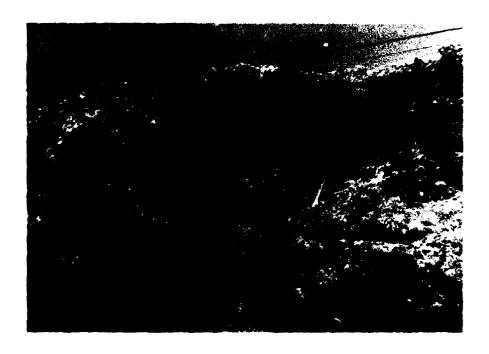
STA 842+00



STA 843+00



STA 844+00



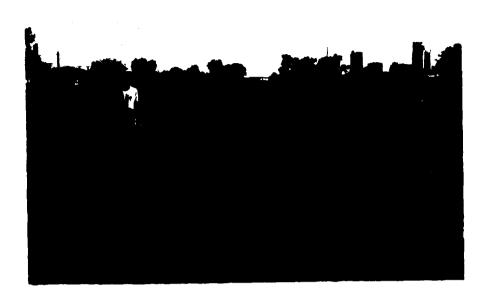
DRAIN TILE STA 841+62 RT



STA 845+00



STA 847+00



STA 848+00



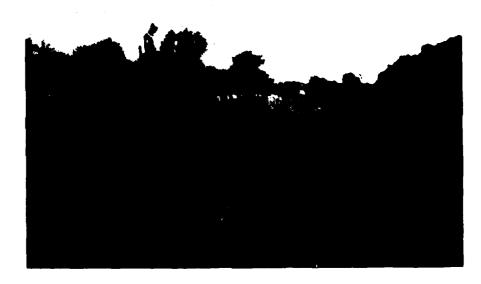
STA 848+00



STA 848+00



STA 849+00



STA 849+00



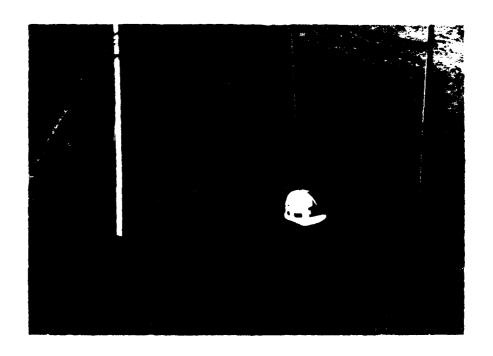
STA 850+00



STA 851+00



STA 852+00



STA 852+84 ROAD "A"



STA 853+00



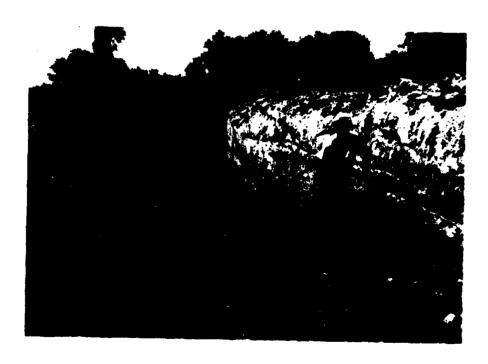
STA 853+00



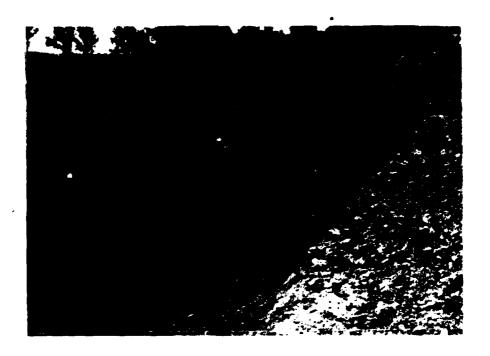
STA 854+00



STA 854+00



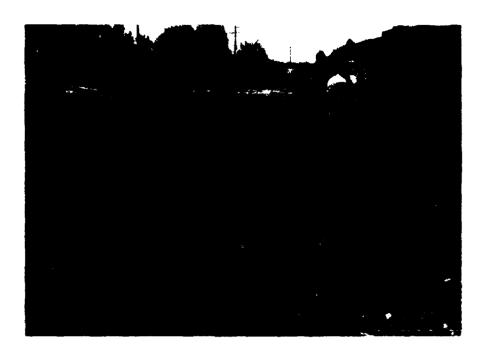
STA 855+00



STA 856+00

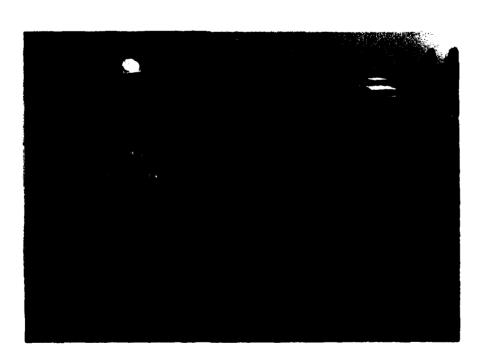


STA 863+00

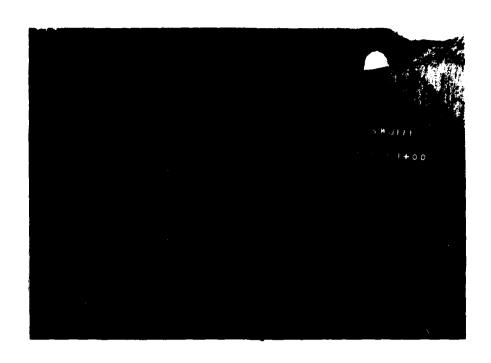


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STA 863+00



STA 864+00



STA 864+00



STA 865+00



STA 865+00



STA 866+00



STA 867+00

